

GENERAL STRUCTURAL NOTES

GENERAL REQUIREMENTS

- DRAWINGS SHOW TYPICAL AND CERTAIN SPECIFIC CONDITIONS ONLY. FOR DETAILS NOT SPECIFICALLY SHOWN PROVIDE DETAILS SIMILAR TO THOSE SHOWN.
- VERIFY ALL EXISTING CONDITIONS, DIMENSIONS AND ELEVATIONS BEFORE STARTING WORK. NOTIFY THE ARCHITECT AND ENGINEER IN WRITING OF ANY DISCREPANCY.
- THE CONTRACTOR IS SOLELY RESPONSIBLE FOR THE DESIGN, ADEQUACY, AND SAFETY OF ERECTION BRACING, SHORING, TEMPORARY SUPPORTS, MEANS AND METHODS, ETC. THE STRUCTURAL ELEMENTS ARE NOT STABLE UNTIL THE STRUCTURE IS COMPLETE.
- COORDINATE AND VERIFY ANY FLOOR AND ROOF OPENINGS, SIZES AND LOCATIONS WITH ARCHITECTURAL, MECHANICAL, PLUMBING, ELECTRICAL DRAWINGS, AND OWNERS EQUIPMENT. FOR ADDITIONAL OPENINGS, INSERTS, SLEEVES, CURBS, PADS, ETC. NOT SHOWN ON THE STRUCTURAL DRAWINGS, SEE CIVIL, ARCHITECTURAL, MECHANICAL, PLUMBING AND ELECTRICAL DRAWINGS.
- REVIEW OF SHOP DRAWINGS AND OTHER SUBMITTALS BY THE ARCHITECT AND ENGINEER DOES NOT RELIEVE THE CONTRACTOR OF THE RESPONSIBILITY TO REVIEW AND CHECK SHOP DRAWINGS BEFORE SUBMITTAL TO THE ENGINEER. THE CONTRACTOR REMAINS SOLELY RESPONSIBLE FOR ERRORS AND OMISSIONS ASSOCIATED WITH THE DIMENSIONS SPECIFIED IN THE CONTRACT DOCUMENTS. CONTRACTOR IS ALSO RESPONSIBLE FOR THE MEANS, METHODS, TECHNIQUES, SEQUENCES, AND PROCEDURES OF CONSTRUCTION.
- DO NOT SCALE DRAWINGS. FOLLOW DIMENSIONS SHOWN ON PLANS OR OBTAIN ADDITIONAL INFORMATION IN WRITING FROM THE ARCHITECT.
- WHERE A SECTION, TYPICAL SECTION, DETAIL, TYPICAL DETAIL OR PLAN NOTE IS SHOWN FOR ONE CONDITION, IT SHALL APPLY TO ALL LIKE OR SIMILAR CONDITIONS UNLESS NOTED OTHERWISE.
- THE CONTRACTOR SHALL COMPLY WITH ALL LOCAL, STATE, FEDERAL AND OSHA REGULATIONS.

PRIMARY CODES AND SPECIFICATIONS

- REFERENCE TO CODES AND STANDARD SPECIFICATIONS OF ANY TECHNICAL ORGANIZATION OR ASSOCIATION, OR TO CODES OF LOCAL OR STATE AGENCIES SHALL MEAN THE LATEST EDITION OF CODE OR SPECIFICATION ADOPTED AT THE TIME THE PROJECT IS TO BE CONSTRUCTED UNLESS APPROVED OTHERWISE IN WRITING THE ENFORCING AGENCY.
- GENERAL BUILDING CODE:
 - INTERNATIONAL BUILDING CODE, 2012 EDITION WITH GEORGIA ADMENDMENTS.
- CONCRETE CODES:
 - ACI 318-09 BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE.
 - ACI 301, LATEST EDITION, SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS.
 - LATEST EDITION OF CRSI MANUAL OF STANDARD PRACTICE & ALL SUPPLEMENTS.
- STRUCTURAL STEEL CODES:
 - AISC 360-10 SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS.
 - AISC 303-10 CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES.
 - SPECIFICATION FOR STRUCTURAL JOINTS USING HIGH-STRENGTH BOLTS, DECEMBER 31, 2009.
- CONCRETE MASONRY:
 - ACI 530-05 BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES.
 - ACI 530-1-05 SPECIFICATIONS FOR MASONRY STRUCTURES.

DESIGN LOADS

- GRAVITY LOADS (REFERENCE ASCE 7-10):

ROOF DEAD LOAD	15 PSF	
ROOF LIVE LOAD	20 PSF	
FLOOR DEAD LOAD - PRECAST PLANKS:	90 PSF	
FLOOR DEAD - WOOD TRUSSES:	25 PSF	
FLOOR LIVE LOAD:	40 PSF	
GROUND FLOOR DEAD LOAD:	100 PSF	
GROUND FLOOR LIVE LOAD:	150 PSF	
- WIND LOAD (REFERENCE ASCE 7-10):

BASIC WIND SPEED, 3 SEC GUST	Vult = 135 MPH	(FIGURE 26.5-1B)
	Vascl = 106 MPH	
RISK CATEGORY	II	(TABLE 1.5-1)
EXPOSURE CATEGORY	C	(SECTION 26.7.3)
INTERNAL PRESSURE COEFFICIENT	GCPi = +/- 0.18	(TABLE 26.11-1)

IN ACCORDANCE WITH ASCE 7-10 THIS STRUCTURE IS NOT LOCATED IN A WIND-BORNE DEBRIS REGION.
- SNOW LOAD (REFERENCE ASCE 7-10):

GROUND SNOW LOAD	Pg = 5 PSF	(FIGURE 7-1)
EXPOSURE FACTOR, TERRAIN CATEGORY B	Ce = 0.90	(TABLE 7-2)
THERMAL FACTOR	Ct = 1.0	(TABLE 7-3)
IMPORTANCE FACTOR	I = 1.0	(TABLE 1.5-2)
- SEISMIC LOAD (REFERENCE ASCE 7-10):

RISK CATEGORY	II	(TABLE 1.5-1)
SOIL SITE CLASSIFICATION	D	(SECTION 11.4.2)
IMPORTANCE FACTOR	I = 1.0	(TABLE 1.5-2)
SPECTRAL RESPONSE AT SHORT PERIOD	Sds = 0.321g	
SPECTRAL RESPONSE AT 1 SEC.	Sd1 = 0.185g	
SEISMIC DESIGN CATEGORY	SDC = C	
SEISMIC FORCE RESISTING SYSTEM	STEEL SYSTEMS NOT SPECIFICALLY DESIGNED FOR SEISMIC RESISTANCE	
DETAILED RESPONSE MODIFICATION FACTOR	R = 3.0	
SEISMIC RESPONSE COEFFICIENT	Cs = 0.107	
DESIGN BASE SHEAR	V = Cs x W	

FOUNDATION SUBSURFACE PREPARATION

- UNLESS NOTED OTHERWISE IN THE DRAWINGS, THE LIMITS OF THIS SUBSURFACE REPARATION ARE CONSIDERED TO BE THAT PORTION OF THE SITE DIRECTLY BENEATH AND 10 FEET BEYOND THE BUILDING AND ITEMS ATTACHED TO THE BUILDING PROPER.
- ALL SUBSURFACE PREPARATION PROCEDURES SHALL BE PERFORMED UNDER THE OBSERVATION OF AN APPROVED TESTING LABORATORY SUPERVISED BY A LICENSED PROFESSIONAL ENGINEER.
- CONTRACTOR SHALL REMOVE ALL EXISTING FOUNDATIONS, SLABS, PAVEMENTS AND BELOW-GRADE STRUCTURES THAT ARE LOCATED WITHIN THE LIMITS OF SUBSURFACE PREPARATION.
- CONTRACTOR SHALL STRIP AND REMOVE ALL SURFACE VEGETATION, TOPSOIL, ROOT SYSTEMS, ORGANIC MATERIAL, AND SOFT OR OTHERWISE UNSUITABLE MATERIAL FROM THE BUILDING AREA. THE DEPTH OF STRIPPING SHALL BE THAT REQUIRED TO REMOVE SIGNIFICANT ROOT ZONES, TREE STUMPS AND OTHER UNACCEPTABLE MATERIALS, BUT IN NO CASE SHALL THE DEPTH OF STRIPPING BE LESS THAN 24".
- COMPACT THE UPPER 24" OF EXPOSED SUBGRADE TO A MINIMUM DENSITY OF 95% MODIFIED PROCTOR (ASTM D 1557) BY PROOFROLLING THE EXPOSED SUBGRADE IN OVERLAPPING PASSES WITH A PNEUMATIC TIRE TANDEM AXLE DUMP TRUCK WEIGHING AT LEAST 20 TONS OR OTHER APPROVED DEVICE. REMOVE AND REPLACE UNSUITABLE AREAS WHICH DO NOT STABILIZE AFTER SUCCESSIVE PASSES OF PROOFROLLING EQUIPMENT AS DIRECTED BY THE GEOTECHNICAL ENGINEER.
- PLACE STRUCTURAL FILL IN THE PREPARED AREA IN 8" TO 10" LIFTS. COMPACT EACH LIFT TO A MINIMUM DENSITY OF 95% MODIFIED PROCTOR (ASTM D 1557). MATERIAL USED AS STRUCTURAL FILL SHALL BE NON-PLASTIC GRANULAR MATERIAL CONTAINING LESS THAN 25% FINES PASSING THROUGH THE NO. 200 SIEVE AND BE FREE OF ORGANICS, ROOTS, OR OTHER DELETERIOUS MATERIALS. MOISTURE CONTENT FOR GRANULAR FILL MATERIAL SHALL BE WITHIN +/- 3% OF THE OPTIMUM MOISTURE CONTENT AS DETERMINED BY THE MODIFIED PROCTOR TEST AT THE TIME OF PLACEMENT AND COMPACTION.
- PREVENT SURFACE WATER AND GROUND WATER FROM ENTERING EXCAVATIONS, FROM PONDING ON PREPARED SUBGRADE AND FROM FLOODING THE PROJECT SITE AND SURROUNDING AREA.
- NOTIFY IMMEDIATELY THE OWNER'S REPRESENTATIVE AND ENGINEER IF UNUSUAL SOIL CONDITIONS ARE FOUND.
- DO NOT ALLOW STORED EXCAVATION MATERIAL TO DISRUPT PROPER DRAINAGE OF AREA.
- DISPOSE OF EXCAVATED MATERIAL AS REQUIRED BY OWNER'S REPRESENTATIVE.

REINFORCED CONCRETE

- ALL CONCRETE WORK SHALL CONFORM TO ACI 301, "SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS", LATEST EDITION. DESIGN IS BASED ON ACI 318, "BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE", LATEST EDITION.
- UNLESS NOTED OTHERWISE ALL CONCRETE SHALL BE NORMAL WEIGHT AND HAVE THE FOLLOWING MINIMUM 28 DAY STRENGTHS:

FOUNDATIONS	3,500 PSI
SLAB-ON-GRADE	4,000 PSI
- CONCRETE MIX DESIGNS, IN ACCORDANCE WITH ACI 318 SECTION 5.3, SHALL BE SUBMITTED TO THE ENGINEER AND TESTING AGENCY FOR MIX DESIGN APPROVAL. RESPONSIBILITY FOR OBTAINING THE REQUIRED CONCRETE DESIGN STRENGTH IS THE CONTRACTORS.
- USE OF CALCIUM CHLORIDE CHLORIDE IONS, OR OTHER SALTS IN CONCRETE IS NOT PERMITTED.
- THE AIR CONTENT IN ALL CONCRETE EXPOSED TO WEATHER SHALL BE BETWEEN 3% AND 6%.
- DETAIL CONCRETE REINFORCEMENT AND ACCESSORIES IN ACCORDANCE WITH ACI 315 "DETAILING MANUAL", LATEST EDITION. SUBMIT SHOP DRAWINGS FOR APPROVAL SHOWING ALL FABRICATION DIMENSIONS AND LOCATIONS FOR PLACING REINFORCING STEEL AND ACCESSORIES. DO NOT BEGIN FABRICATION UNTIL SHOP DRAWINGS ARE COMPLETED AND APPROVED.
- DETAIL ALL CONCRETE WALLS AND BEAMS IN ELEVATION UNLESS SPECIFICALLY APPROVED OTHERWISE. CUT SECTIONS SHOWING BAR LOCATIONS AND CONCRETE COVER.
- REINFORCING STEEL SHALL CONFORM TO ASTM A615, GRADE 60 UNLESS NOTED OTHERWISE.
- ALL REINFORCING STEEL AND EMBEDDED ITEMS SHALL BE TIED SECURELY IN PLACE PRIOR TO PLACING CONCRETE. PROVIDE SUFFICIENT SUPPORTS TO MAINTAIN POSITION OF REINFORCEMENT WITHIN SPECIFIED TOLERANCES DURING ALL CONSTRUCTION ACTIVITIES. STICKING DOWELS INTO WET CONCRETE IS NOT PERMITTED.
- PROVIDE CONTINUOUS REINFORCEMENT WHEREVER POSSIBLE. SPLICE ONLY AS SHOWN ON DRAWINGS. STAGGER SPLICES WHERE POSSIBLE. LAP ALL SPLICES 40 BAR DIAMETERS BUT NOT LESS THAN 2'-6" UNLESS DETAILED OTHERWISE. DOWELS SHALL MATCH THE SIZE AND SPACING OF THE SPECIFIED REINFORCEMENT OVER AND SHALL BE LAPPED 40 BAR DIAMETERS BUT NOT LESS THAN 2'-6" UNLESS DETAILED OTHERWISE.
- REINFORCING STEEL SHALL HAVE THE FOLLOWING CONCRETE COVER UNLESS NOTED OTHERWISE:

CONCRETE CAST AGAINST EARTH (NOT FORMED)	3"
FORMED CONCRETE EXPOSED TO EARTH OR WEATHER	
#6 THROUGH #18 BARS	2"
#5 BARS AND SMALLER	1 1/2"
CONCRETE NOT EXPOSED TO EARTH OR WEATHER	
SLABS, JOISTS AND WALLS	1"
BEAMS (STIRRUPS)	1 1/2"
- DO NOT PLACE PIPES OR DUCTS EXCEEDING ONE-THIRD THE SLAB OR WALL THICKNESS WITHIN THE SLAB UNLESS SPECIFICALLY SHOWN AND DETAILED ON THE STRUCTURAL DRAWINGS.

REINFORCED CONCRETE MASONRY

- UNLESS NOTED OTHERWISE, PROVIDE HOLLOW, LOAD BEARING CONCRETE MASONRY UNITS (CMU) CONFORMING TO ASTM C90, TYPE 1, WITH A MAXIMUM DENSITY OF 105 PCF.
- PROVIDE CONCRETE MASONRY WITH A MINIMUM COMPRESSIVE STRENGTH $f'm = 1,500$ PSI CORRESPONDING TO A UNIT STRENGTH OF 2,000 PSI ON NET CROSS SECTIONAL AREA OF CMU DETERMINED IN ACCORDANCE WITH ASTM C140.
- PROVIDE TYPE "S" MORTAR IN ACCORDANCE WITH ASTM C270, UNLESS NOTED OTHERWISE. MORTAR BED JOINTS SHALL NOT EXCEED 5/8" THICKNESS.
- PROVIDE GROUT FOR REINFORCED MASONRY IN ACCORDANCE WITH ASTM C476 WITH A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI UNLESS NOTED OTHERWISE.
- LAP REINFORCING BARS 48 BAR DIAMETERS AT SPLICES UNLESS DETAILED OTHERWISE.
- PROVIDE TRUSS OR LADDER TYPE HORIZONTAL JOINT REINFORCEMENT COMPLYING WITH ASTM A82, WITH A MINIMUM OF (2) NO. 9 LONGITUDINAL LINES, ZINC COATED, PLACED AT 16" O.C. UNLESS NOTED OTHERWISE.
- LAY MASONRY UNITS IN RUNNING BOND PATTERN UNLESS NOTED OTHERWISE.
- SIDES AND TOPS OF MASONRY WALL PANELS SHALL BE ANCHORED TO STRUCTURE BY DOVETAIL ANCHORS, METAL STRAPS, OR EQUIVALENT UNLESS NOTED OTHERWISE.
- BOND BEAMS, CMU LINTELS, AND OTHER STRUCTURAL ELEMENTS SHALL EXTEND UNINTERRUPTED ACROSS CONTROL JOINTS. PROVIDE RAKED JOINTS IN THESE ELEMENTS TO MATCH THE CONTROL JOINT.
- PROVIDE 8" DEEP BOND BEAM REINFORCED WITH 2#5 CONTINUOUS AS FOLLOWS:
 - AT ALL WINDOW AND DOOR HEADS
 - AT 48" O.C.
 - AT TOP OF ALL WALLS

STRUCTURAL STEEL

- ALL STRUCTURAL STEEL CONSTRUCTION SHALL CONFORM TO AISC 360-05 "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS", AND AISC 303-05 THE CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES.
- SHOP DRAWINGS PREPARED IN ACCORDANCE WITH THE LATEST "STRUCTURAL STEEL DETAILING MANUAL" OF THE AISC SHALL BE SUBMITTED FOR APPROVAL. NO FABRICATION SHALL BEGIN UNTIL SHOP DRAWINGS ARE COMPLETED AND APPROVED.
- UNLESS NOTED OTHERWISE, STRUCTURAL STEEL SHALL CONFORM TO ASTM A572, GRADE 50, CHANNELS, ANGLES AND PLATES MAY BE ASTM A36 UNLESS NOTED OTHERWISE. SQUARE AND RECTANGULAR TUBES SHALL CONFORM TO ASTM A500, GRADE B. ROUND PIPES SHALL CONFORM TO ASTM A53, GRADE B.
- STEEL FRAMING CONNECTIONS SHALL BE BOLTED OR WELDED:
 - BOLTED JOINTS SHALL CONFORM TO AISC "SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS". BOLTS SHALL CONFORM TO ASTM A325, AND SHALL BE MINIMUM 3/4" DIAMETER, UNLESS NOTED OTHERWISE PROVIDE DIRECT TENSION INDICATORS (LOAD INDICATING WASHERS OR SNAP OFF BOLTS) IN ACCORDANCE WITH ASTM F999 AT ALL HIGH STRENGTH BOLTS.
 - WELDS SHALL CONFORM TO THE "STRUCTURAL WELDING CODE" OF THE AMERICAN WELDING SOCIETY, AWS D1.1. USE E70XX ELECTRODES. WELDING PROCESSES AND OPERATORS SHALL BE QUALIFIED IN ACCORDANCE WITH AWS "STANDARD QUALIFICATIONS PROCEDURES". WELDERS SHALL CARRY PROOF OF QUALIFICATIONS ON THEIR PERSONS.
- ANCHOR BOLTS SHALL CONFORM TO ASTM F 1554 GRADE 36 UNLESS NOTED OTHERWISE.
- DO NOT USE GAS CUTTING TORCHES FOR CORRECTING FABRICATION ERRORS IN THE STRUCTURAL FRAMING.
- PROVIDE (1) SHOP COAT OF STANDARD PRIMER PAINT. PRIMER TO BE COMPATIBLE WITH THE FINISH COAT. TOUCH UP AREAS DAMAGED IN HANDLING AND ERECTION WITH THE SAME PAINT USED FOR THE SHOP COAT. STEEL SURFACES TO BE WELDED OR ENCASED IN CONCRETE OF FIREPROOFING, CONNECTIONS DESIGNED AS SLIP CRITICAL TYPE, OR SURFACES RECEIVING WELDED SHEAR CONNECTIONS IN THE FIELD SHALL NOT BE PAINTED.
- PACK BELOW ALL BASE PLATES WITH NON-SHRINK, NON-METALLIC HI-STRENGTH GROUT (MINIMUM 6,000 PSI) AFTER SETTING AND LEVELING.
- PROVIDE TEMPORARY BRACING OF STRUCTURAL FRAMING UNTIL ALL PERMANENT BRACING, MOMENT CONNECTIONS, AND ROOF DECKS (DIAPHRAGMS) ARE COMPLETELY INSTALLED. THE STRUCTURAL ELEMENTS ARE UNSTABLE UNTIL THE STRUCTURE IS COMPLETE IN ACCORDANCE WITH THE PLANS.

TIMBER FRAMING

- LOCATION, NUMBER, AND DIMENSIONS OF FRAMING MEMBERS SHOW GENERAL ARRANGEMENT ONLY. ACTUAL SPANS, SPACING, ETC. SHALL BE DETERMINED FROM ARCHITECTURAL DETAILS.
- SEE ARCHITECTURAL PLANS AND DETAILS FOR EDGE SECTIONS, HEADER AND LINTEL LOCATIONS, AND ALL NON-STRUCTURAL FRAMING AND TRIM.
- PROVIDE NAILING PATTERN IN COMPLIANCE WITH IBC RECOMMENDED FASTENING SCHEDULE. NAILS SHALL BE IN ACCORDANCE WITH MINIMUM NAILING REQUIREMENTS OF IBC 2012 EXCEPT WHERE NOTED. ALL NAILS SHALL BE HOT-DIPPED GALVANIZED.
- LOAD BEARING STUD WALLS SHALL BE CONTINUOUSLY BRIDGED AT MAXIMUM 48" O.C. ALONG ALL EDGES OF GYPSUM BOARD, AND ALL UNSUPPORTED PLYWOOD WALL SHEATHING JOINTS WITH SOLID WOOD BLOCKING SAME NOMINAL SIZE AS WALL STUDS.
- NO CUTS, HOLES, OR COPES IN STRUCTURAL WOOD FRAMING SHALL BE PERMITTED WITHOUT PRIOR REVIEW AND APPROVAL BY THE STRUCTURAL ENGINEER AND ARCHITECT.
- ALL DIMENSIONAL LUMBER SHALL BE #2 KYD OR BETTER AND PROVIDE NOT LESS THAN THE MINIMUM DESIGN VALUES AS PUBLISHED BY THE NATIONAL DESIGN SPECIFICATION, LATEST EDITION.
- WOOD PANEL SHEATHING SHALL BE AS FOLLOWS:

WALLS	APA RATED SHEATHING, EXPOSURE 1 OR EXTERIOR
ROOF	APA RATED SHEATHING, EXPOSURE 1, 2, OR EXTERIOR
- ALL BOLTS SHALL BE GALVANIZED ASTM A307 WITH PLATE WASHERS, GALVANIZED.

PRE-ENGINEERED WOOD TRUSSES

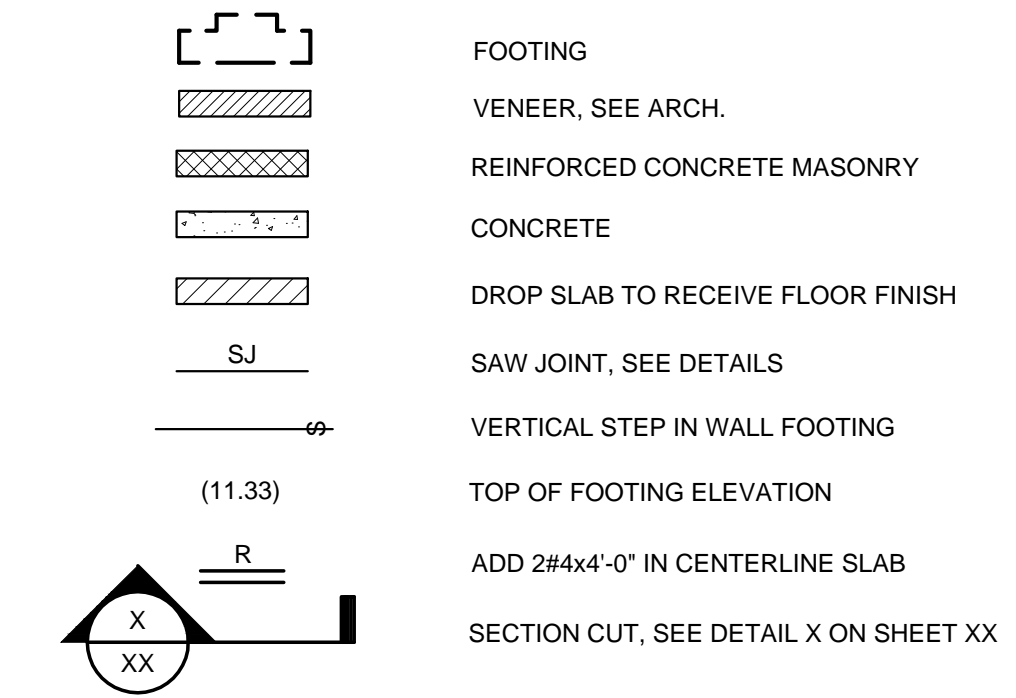
- WOOD TRUSSES SHALL CONFORM TO THE MOST CURRENT VERSION OF THE DESIGN SPECIFICATION FOR LIGHT METAL PLATE CONNECTED WOOD ROOF AND FLOOR TRUSSES BY THE TRUSS PLATE INSTITUTE (TPI) AND THE NATIONAL DESIGN SPECIFICATIONS FOR STRESS GRADE LUMBER AND ITS FASTENING BY THE NATIONAL FOREST PRODUCTS ASSOCIATION. ADDITIONAL DESIGN AND DETAILING REQUIREMENTS SET FORTH IN THIS SECTION SHALL BE PROVIDED BY THE TRUSS MANUFACTURER.
- ALL ROOF TRUSSES SHALL BE DESIGNED, FABRICATED, AND ERECTED TO SUPPORT THE FOLLOWING MINIMUM LOADS:

LIVE - TOP CHORD	- 20 PSF
DEAD - TOP CHORD	- 10 PSF + MECHANICAL EQUIPMENT
DEAD - BOTTOM CHORD	- 10 PSF + MECHANICAL EQUIPMENT
- PROVIDE AT EACH SUPPORT GALVANIZED METAL HURRICANE PLATE ANCHORAGES SUFFICIENT TO RESIST HORIZONTAL AND VERTICAL WIND COMPONENTS WITH A SAFETY FACTOR OF 3. MINIMUM UPLIFT LOAD SHALL DETERMINED BY TRUSS MANUFACTURER FROM REQUIREMENTS OF LOCAL BUILDING CODES.
- INSTALL TEMPORARY AND PERMANENT VERTICAL BRACING OR OTHER BRACES AS RECOMMENDED BY THE TRUSS MANUFACTURER AND/OR APPLICABLE REFERENCES.
- SHOP DRAWINGS, COMPUTATIONS, ETC. SHALL BE SUBMITTED FOR REVIEW. SHOP DRAWINGS SHALL PROVIDE ERECTION LAYOUT FOR TRUSS RAFTERS, OUTRIGGERS, HEADERS, BRACING, ETC. SEE ARCHITECTURAL AND STRUCTURAL PLANS FOR SUPPORT LOCATIONS.
- CALCULATIONS SHALL BEAR THE SEAL OF A REGISTERED PROFESSIONAL ENGINEER LICENSED IN THE STATE OF GEORGIA.
- TRUSS WEB TO CHORD CONNECTION SHALL BE MADE WITH GALVANIZED STEEL TRUSS CONNECTION PLATES MEETING ALL REQUIREMENTS OF THE TRUSS PLATE INSTITUTE.
- ROOF SHEATHING SHALL BE MINIMUM 5/8" APA RATED TAG SHEATHING GLUED AND FASTENED IN ACCORDANCE WITH THE MINIMUM NAILING REQUIREMENTS OF IBC 2012 UNLESS DETAILED OTHERWISE.

STRUCTURAL ABBREVIATIONS

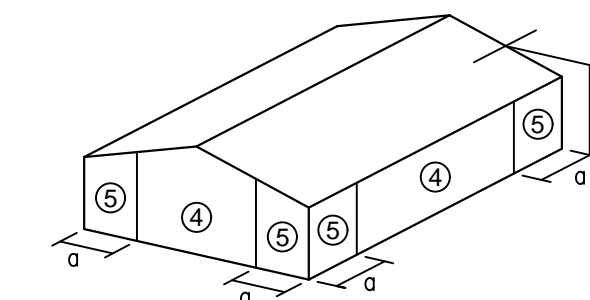
O.C.	ON CENTER	DIA.	DIAMETER
T.O.S.	TOP OF STEEL	PL.	PLATE
T.O.F.	TOP OF FOOTING	CLR.	CLEAR
FIN. FLR.	FINISHED FLOOR	T&B	TOP & BOTTOM
U.N.O.	UNLESS NOTED OTHERWISE	PLF	POUNDS PER LINEAR FOOT
P.T.	PRESSURE TREATED	PSF	POUNDS PER SQUARE FOOT
GALV.	GALVANIZED	K	KIPS
WF	WIDE FLANGE	KSI	KIPS PER SQUARE INCH
C	CHANNEL	LBS.	POUNDS
L	ANGLE	IN.	INCH
L.L.V.	LONG LEG VERTICAL	FT.	FEET
L.L.H.	LONG LEG HORIZONTAL	TYP.	TYPICAL
HSS	HOLLOW STRUCTURAL SECTION	JST.	JOIST
COL	COLUMN	PC.	PIECE
BP	BASE PLATE	SYP	SOUTHERN YELLOW PINE
AB	ANCHOR BOLT	DL	DEAD LOAD
FTG.	FOOTING	LL	LIVE LOAD
ARCH.	ARCHITECTURAL	WL	WIND LOAD
HORIZ.	HORIZONTAL	GB	GRADE BEAM
VERT.	VERTICAL	GA	GAUGE
CJ	CONTROL JOINT	WWF	WELDED WIRE FABRIC
E.F.	EACH FACE	MAX	MAXIMUM
E.W.	EACH WAY	MIN.	MINIMUM
REINF.	REINFORCEMENT	LG	LONG
SJ	SAW JOINT	ADDL.	ADDITIONAL
CONC.	CONCRETE	P.A.F.	POWDER ACTUATED FASTENER
ELEV.	ELEVATION	Ø	DIAMETER

STRUCTURAL SYMBOLS and NOTATIONS



DESIGN PRESSURE RATING SCHEDULE

MEAN ROOF HEIGHT	* EXPOSURE B * INCLUDING INLAND PROPERTY ONLY		* EXPOSURE C * INCLUDING ISLANDS AND WATER/MARSH FRONT PROPERTY		* EXPOSURE D * INCLUDING ISLANDS AND WATER/MARSH FRONT PROPERTY	
	ZONE (4)	ZONE (5)	ZONE (4)	ZONE (5)	ZONE (4)	ZONE (5)
15'	DP 35	DP 45	DP 40	DP 50	DP 50	DP 60
20'	DP 35	DP 45	DP 45	DP 55	DP 50	DP 65
25'	DP 35	DP 45	DP 45	DP 55	DP 55	DP 65
30'	DP 35	DP 45	DP 50	DP 60	DP 55	DP 70
35'	DP 35	DP 45	DP 50	DP 60	DP 55	DP 70

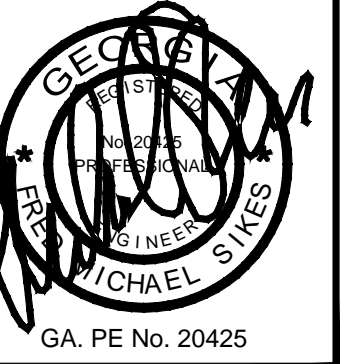


- NOTES:**
- DESIGN PRESSURE (DP) VALUES ARE LISTED IN POUNDS/SQUARE FOOT (PSF).
10-11-17
 - h = MEAN ROOF HEIGHT AND IS MEASURED FROM GRADE TO HALFWAY UP THE ROOF.
 - a = LENGTH OF ZONE (5) LOCATED AT THE CORNERS OF THE BUILDING A DISTANCE OF THE FOLLOWING CRITERIA:
THE SMALLER OF THE FOLLOWING:
 - 10% OF THE LEAST HORIZONTAL DIMENSION
 - OR, 4h
 B. BUT NOT LESS THAN THE LARGER OF EITHER OF THE FOLLOWING:
 - 4% OF THE LEAST HORIZONTAL DIMENSION
 - OR, 3 FEET

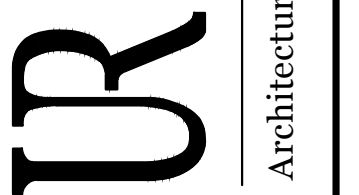
1 WINDOW AND DOOR DESIGN PRESSURE (DP) RATING

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USSERY/RULE ARCHITECTS P.C.
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9-7-17	
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CAST-IN -PLACE AUGERED PILES

GENERAL

1. THE GENERAL PROVISIONS OF THE CONTRACT, INCLUDING GENERAL AND SUPPLEMENTARY CONDITIONS AND GENERAL REQUIREMENTS APPLY TO THE WORK SPECIFIED IN THIS SECTION.
2. THE EXTENT OF CAST-IN-PLACE AUGERED PILING IS SHOWN ON THE DRAWINGS, INCLUDING LOCATIONS, DIAMETERS, ESTIMATED BOTTOM ELEVATIONS, TOP ELEVATIONS, REINFORCING, AND DETAILS OF CONSTRUCTION. ONE (1) TEST PILE SHALL BE INCLUDED IN THE WORK.

QUALITY ASSURANCE

1. PERFORM WORK IN COMPLIANCE WITH THE APPLICABLE REQUIREMENTS OF GOVERNING AUTHORITIES HAVING JURISDICTION, INCLUDING PROVISIONS FOR ADEQUATE PROTECTION TO PERSONS AND PROPERTY. APPLICABLE PROVISIONS OF THE 2012 EDITION OF THE INTERNATIONAL BUILDING CODE WITH GEORGIA AMENDMENTS SHALL BE INCLUDED AS PART OF THIS SECTION.
2. ENGAGE A REGISTERED SURVEYOR OR REGISTERED CIVIL ENGINEER ACCEPTABLE TO THE ARCHITECT TO PERFORM ALL SURVEYS, LAYOUTS, AND MEASUREMENTS FOR PILE WORK. THE SURVEYOR SHALL CONDUCT THE LAYOUT WORK FOR EACH PILE TO THE LINES AND LEVELS REQUIRED BEFORE EXCAVATION, AND SUBMIT TO ARCHITECT FOR APPROVAL THE ACTUAL MEASUREMENTS OF EACH PILE'S HORIZONTAL AXIAL LOCATION, DIAMETER, BOTTOM AND TOP ELEVATIONS, DEVIATIONS FROM SPECIFIED TOLERANCES, AND OTHER DATA AS REQUIRED PRIOR TO ACCEPTANCE OR CONSTRUCTION OF PILE CAPS AND GRADE BEAMS.
3. THE SURVEYOR SHALL RECORD AND MAINTAIN ALL INFORMATION PERTINENT TO EACH PILE AND COOPERATE WITH TESTING AND INSPECTION PERSONNEL TO PROVIDE DATA FOR REQUIRED REPORTS.
4. EMPLOY, AT OWNERS'S EXPENSE, A TESTING LABORATORY ACCEPTABLE TO THE ARCHITECT TO PERFORM MATERIALS EVALUATION TEST AND TO DESIGN GROUT MIXES.
5. MATERIALS AND INSTALLED WORK MAY REQUIRE TESTING AND RETESTING, AS DIRECTED BY THE ARCHITECT, AT ANY TIME DURING THE PROGRESS OF THE WORK. ALLOW FREE ACCESS TO MATERIAL STOCKPILES AND FACILITIES AT ALL TIMES. TESTS, NOT SPECIFICALLY INDICATED TO BE DONE AT THE OWNER'S EXPENSE, INCLUDING THE RETESTING OF REJECTED MATERIALS AND INSTALLED WORK, SHALL BE DONE AT THE CONTRACTOR'S EXPENSE.
6. SUBMIT WRITTEN REPORTS TO THE ARCHITECT, FOR EACH MATERIAL SAMPLED AND TESTED PRIOR TO THE START OF WORK. PROVIDE THE PROJECT IDENTIFICATION NAME AND NUMBER, DATE OF REPORT, NAME OF CONTRACTOR NAME OF CONCRETE TESTING SERVICE, SOURCE OF CONCRETE AGGREGATES, MATERIAL MANUFACTURER AND BRAND NAME FOR MANUFACTURER MATERIALS, VALUES SPECIFIED IN THE REFERENCED SPECIFICATION FOR EACH MATERIAL, AND TEST RESULTS. INDICATE WHETHER OR NOT MATERIAL IS ACCEPTABLE FOR INTENDED USE.
7. A REGISTERED PROFESSIONAL ENGINEER EMPLOYED BY A RECOGNIZED AND APPROVED GEOTECHNICAL ENGINEERING FIRM HEREIN REFERRED TO AS THE 'SOIL ENGINEER' SHALL SUPERVISE THE INSTALLATION OF THE PILES. COSTS SHALL BE PAID BY THE OWNER.
8. PILES SHALL BE INSTALLED BY AN EXPERIENCED CONTRACTOR WHO SHALL HAVE A MINIMUM OF 5 YEARS CONTINUOUS EXPERIENCE IN THE INSTALLATION OF AUGERED CAST-IN-PLACE PILES. EVIDENCE OF EXPERIENCE SHALL BE SUBMITTED TO THE ARCHITECT FOR REVIEW AND SHALL INCLUDE THE NAMES OF NO LESS THAN 10 PROJECTS, EQUAL TO OR GREATER IN SCOPE THAN THIS PROJECT. PROJECT MANAGER AND FIELD SUPERINTENDENT FOR PILE INSTALLER MUST EACH HAVE NO LESS THAN 3 YEARS EXPERIENCE IN THE INSTALLATION OF AUGERED CAST-IN-PLACE PILES UNDER SIMILAR SUBSURFACE SOIL CONDITIONS.

SUBMITTALS

1. SUBMIT 3 COPIES OF THE FOLLOWING REPORTS DIRECTLY TO THE ARCHITECT FOR REVIEW:
 - A. MIX DESIGN PROPOSED FOR USE IN PILING.
 - B. VERIFIED PILE REPORT AND INSTALLATION LOG FOR EACH PILE, RECORDING THE FOLLOWING MINIMUM DATA:
 1. DATE
 2. IDENTIFICATION OF THE INDIVIDUAL PILE AND PILE GROUP
 3. DIAMETER AND REINFORCING INSTALLED
 4. FINAL PILE TRIP ELEVATION
 5. BUTT ELEVATION
 6. THEORETICAL GROUT VOLUME
 7. ACTUAL GROUT VOLUME
 8. GROUT RATIO
 9. PRESSURE AT PUMP
 10. PRESSURE AT OPERATOR
 11. COMMENTS ON ANYTHING UNUSUAL RELATIVE TO THE INSTALLATION
 THE INSTALLATION LOG SHALL BECOME THE BASIS FOR FINAL PAYMENT OF PILING. IT SHALL BE SUBMITTED TO THE ARCHITECT IN A FORM APPROVED BY HIM, AND SHALL BE VERIFIED BY THE SUPERINTENDENT IN CHARGE OF PILE DRIVING OPERATIONS AND COUNTERSIGNED BY THE CONTRACTOR AND SOIL ENGINEER.
 - C. GROUT STRENGTH TEST REPORTS, RECORDING ALL PERTINENT INFORMATION AND CERTIFICATION FOR COMPLIANCE WITH PROJECT REQUIREMENTS BASED ON FIELD SAMPLING.
 - D. REINFORCING SHOP DRAWINGS, FOR APPROVAL PRIOR TO FABRICATION.
 - E. LOAD-BEARING TEST REPORTS SHALL BE SUBMITTED FOR REVIEW FOR EACH LOAD-BEARING TEST WITHIN 2 DAYS AFTER COMPLETION OF TESTS.

JOB CONDITIONS

1. THE DATA ON INDICATED SUBSURFACE CONDITIONS IS NOT INTENDED AS REPRESENTATIONS OR WARRANTIES OF THE CONTINUITY OF SUCH CONDITIONS. IT IS EXPRESSLY UNDERSTOOD THAT THE OWNER WILL NOT BE RESPONSIBLE FOR INTERPRETATIONS OR CONCLUSIONS DRAWN THEREFROM BY THE CONTRACTOR. DATA IS MADE AVAILABLE FOR THE CONVENIENCE OF THE CONTRACTOR AND IS NOT GUARANTEED TO REPRESENT ALL CONDITIONS THAT MAY BE ENCOUNTERED. ADDITIONAL TEST BORINGS AND OTHER EXPLORATORY OPERATIONS MAY BE MADE BY THE CONTRACTOR AT NO ADDITIONAL COST TO THE OWNER.
2. LOCATE EXISTING UNDERGROUND UTILITIES BY HAND EXCAVATION BEFORE COMMENCING AUGERED PILE EXCAVATION OPERATIONS. IF UTILITIES ARE TO REMAIN IN PLACE, PROVIDE PROTECTION FROM DAMAGE DURING AUGERED PILE OPERATIONS.
3. SHOULD UNCHARTED OR INCORRECTLY CHARTED PIPING OR OTHER UTILITIES BE ENCOUNTERED DURING EXCAVATION, CONSULT THE ARCHITECT IMMEDIATELY FOR DIRECTIONS AS TO PROCEDURE. COOPERATE WITH THE OWNER AND PUBLIC OR PRIVATE UTILITY COMPANIES IN KEEPING THEIR RESPECTIVE SERVICE AND FACILITIES IN OPERATION. REPAIR DAMAGED UTILITIES TO THE SATISFACTION OF THE UTILITY OWNER.
4. DO NOT INTERRUPT EXISTING UTILITIES SERVING FACILITIES OCCUPIED AND USED BY THE OWNER AND OTHERS, EXCEPT WHEN PERMITTED IN WRITING BY THE ARCHITECT AND THEN ONLY AFTER ACCEPTABLE TEMPORARY UTILITY SERVICES HAVE BEEN PROVIDED.

PRODUCTS

1. PORTLAND CEMENT: ASTM C 150, TYPE I OR II.
2. WATER: FRESH, CLEAR AND FREE FROM INJURIOUS AMOUNTS OF SEWAGE, OIL, ACID, ALKALI, SALTS OR ORGANIC MATTER.
3. FINE AGGREGATES: ASTM C 33, EXCEPT GRADATION SHALL CONSIST OF HARD, DENSE, DURABLE UNCOATED ROCK PARTICLES FREE FROM INJURIOUS AMOUNTS OF SILT, LOAM LUMPS, SOFT OR FLAKY PARTICLES, SHALE, ALKALI, ORGANIC MATTER, MICA AND OTHER DELETERIOUS SUBSTANCES. IF WASHED, THE METHOD SHALL BE SUCH AS WILL NOT REMOVE DESIRABLE FINES, AND THE SAND SHALL SUBSEQUENTLY BE PERMITTED TO DRAIN UNTIL THE RESIDUAL FREE MOISTURE IS REASONABLY UNIFORM AND STABLE. THE SAND SHALL BE WELL GRADED FROM FINE TO COARSE, WITH FINENESS MODULUS BETWEEN 1.40 AND 3.40. THE FINENESS MODULUS IS DEFINED AS THE TOTAL DIVIDEND BY 100 OF THE CUMULATIVE PERCENTAGES RETAINED ON U.S. STANDARD SIEVE NOS. 16, 30, 50 AND 100.
6. REINFORCING BARS AND TIES: ASTM A 615, GRADE 60.

DESIGN MIX

1. GENERAL: THE GROUT USED TO FILL THE HOLES SHALL CONSIST OF A MIXTURE OF PORTLAND CEMENT, SAND, AND WATER PROPORTIONED AND MIXED AS TO PROVIDE A MIXTURE CAPABLE OF MAINTAINING THE SOLIDS IN SUSPENSION WITHOUT APPRECIABLE WATER GAIN, YET WHICH MAY BE PUMPED WITHOUT DIFFICULTY AND WHICH WILL LATERALLY PENETRATE AND FILL ANY VOIDS IN THE FOUNDATION MATERIAL. THE DESIGN MIX SHALL BE PROPORTIONED TO PROVIDE A HARDENED GROUT OF 4,000 PSI (MIN), COMPRESSIVE STRENGTH AT 28 DAYS.
2. SUBMIT WRITTEN REPORTS TO THE ARCHITECT OF PROPOSED MIX FOR EACH CLASS OF CONCRETE AT LEAST 15 DAYS PRIOR TO START OF WORK. DO NOT BEGIN PRODUCTION UNTIL MIXES HAVE BEEN REVIEWED BY ARCHITECT.
3. LABORATORY TRIAL BATCHES: WHEN LABORATORY TRIAL BATCHES ARE USED TO SELECT PROPORTIONS, PREPARE AND TEST IN ACCORDANCE WITH ASTM C109. ESTABLISH A CURVE SHOWING RELATIONSHIP BETWEEN WATER-CEMENT RATIO (OR CEMENT CONTENT) AND COMPRESSIVE STRENGTH, WITH AT LEAST 3 POINTS REPRESENTING BATCHES WHICH PRODUCE STRENGTHS ABOVE AND BELOW THAT REQUIRED. USE NOT LESS THAN 3 SPECIMENS TESTED AT 28-DAYS, OR AN EARLIER AGE WHEN ACCEPTABLE TO THE ARCHITECT, TO ESTABLISH EACH POINT ON THE CURVE.
4. ADJUSTMENT TO CONCRETE MIXING: MIX DESIGN ADJUSTMENTS MAY BE REQUIRED BY CONTRACTOR WHEN CHARACTERISTICS OF MATERIALS, JOB CONDITIONS, WEATHER, TEST RESULTS, OR OTHER CIRCUMSTANCES WARRANT. MIX DESIGN ADJUSTMENTS SHALL BE AT NO ADDITIONAL COST TO THE OWNER AND AS ACCEPTED BY THE ARCHITECT. LABORATORY TEST DATA FOR REVISED MIX DESIGNS AND STRENGTH RESULTS SHALL BE SUBMITTED TO AND ACCEPTED BY THE ARCHITECT BEFORE USING IN THE WORK.

MATERIALS AND INSTALLATION

1. GENERAL: PILES SHALL BE AUGERED, HIGH STRENGTH CEMENT GROUTED, CAST-IN-PLACE PILES OF DIAMETER AND ELEVATIONS DESCRIBED AND LOCATED ON THE STRUCTURAL DRAWINGS, AND SHALL BE CAPABLE OF SUPPORTING THE DESIGN WORKING LOADS INDICATED.
2. PILES SHALL BE INSTALLED BY 14" DIAMETER CONTINUOUS-FLIGHT HOLLOW SHAFT AUGER ROTATED TO THE SPECIFIED PILE DEPTH. HIGH STRENGTH GROUT SHALL BE PUMPED AS THE AUGER IS WITHDRAWN TO FILL THE HOLE, PREVENTING HOLE COLLAPSE AND TO CAUSE THE LATERAL PENETRATION OF THE GROUT INTO SOFT OR POROUS ZONES OF THE SURROUNDING SOIL. A HEAD OF 10' (MIN.) SHALL BE CARRIED AROUND THE PERIMETER OF THE AUGER FLIGHTING AT ALL TIMES DURING WITHDRAWAL SO AS TO PROVIDE A DISPLACING ACTION REMOVING ANY LOOSE MATERIAL FROM THE HOLE AHEAD OF THE TIP.
3. THE INSTALLED LENGTH AND DRILLING CRITERIA OF PRODUCTION PILES SHALL BE AS DETERMINED BY THE SOILS ENGINEER BASED ON INFORMATION FROM THE INSTALLATION PROBE PILES AND THE SUBSEQUENT PILE LOAD TESTS. THE RATE OF AUGER WITHDRAWAL, RATE OF GROUT PUMPING, AND OTHER PROCEDURES USED FOR THE INSTALLATION OF THE TEST PILES SHALL BE DUPLICATED, OR MODIFIED IN ACCORDANCE WITH THE SOILS ENGINEER'S RECOMMENDATIONS DEVELOPED FROM THE TEST PROGRAM FOR THE INSTALLATION OF ALL REMAINING PRODUCTION PILES.
4. LOCATION OF PILES AND TOLERANCES: PILES SHALL BE LOCATED AS SHOWN ON THE DRAWINGS. PILE CENTERS SHALL BE LOCATED WITHIN A TOLERANCE NOT TO EXCEED A DEVIATION OF 2" FROM DESIGNATED POSITION FOR A SINGLE PILE NOR MORE THAN A CUMULATIVE DEVIATION OF 3" FOR ANY TWO ADJACENT PILES.
5. ADJACENT PILES SHALL NOT BE PLACED CLOSER THAN 6 DIAMETERS ON CENTER UNTIL THE GROUT IN THE PILES HAS SET FOR 24 HOURS.
6. REINFORCEMENT: REINFORCEMENT SHALL BE PRE-ASSEMBLED AND SET TO DEPTH AND LOCATION AS SHOWN ON DRAWINGS WHILE GROUT IS STILL FLUID.

MIXING AND PUMPING OF HIGH-STRENGTH CEMENT GROUT

1. ONLY APPROVED PUMPING AND CONTINUOUS MIXING EQUIPMENT SHALL BE USED IN THE PREPARATION AND HANDLING OF THE GROUT. ALL OIL OR OTHER RUST INHIBITORS SHALL BE REMOVED FROM MIXING DRUMS AND PRESSURE GROUT PUMPS.
2. THE GROUT PUMP SHALL BE A POSITIVE DISPLACEMENT PISTON TYPE PUMP CAPABLE OF DEVELOPING PRESSURES AT THE PUMP UP TO 350 PSI.
3. THE MINIMUM VOLUME OF GROUT PLACED IN THE HOLE SHALL AT LEAST EQUAL THE VOLUME OF THE AUGERED HOLE. ALL MATERIALS SHALL BE SUCH AS TO PRODUCE A HOMOGENEOUS MORTAR OF THE DESIRED CONSISTENCY. IF THERE IS A LAPSE IN THE OPERATION, THE MORTAR SHALL BE RECIRCULATED THROUGHOUT THE PUMP.

OBSTRUCTIONS

1. SHOULD ANY OBSTRUCTIONS BE ENCOUNTERED WHICH PREVENT PLACING PILES TO THE DEPTH REQUIRED, OR WHICH CAUSE THE PILES TO DRIFT FROM THE REQUIRED LOCATIONS, THE PILE SHALL BE COMPLETED AT THAT DEPTH AT WHICH THE OBSTRUCTION IS ENCOUNTERED, AND THE ARCHITECT SHALL BE NOTIFIED IMMEDIATELY. IF DIRECTED BY THE ARCHITECT, AN ADDITIONAL ADJACENT PILE SHALL BE PLACED BY THE CONTRACTOR.

TESTS

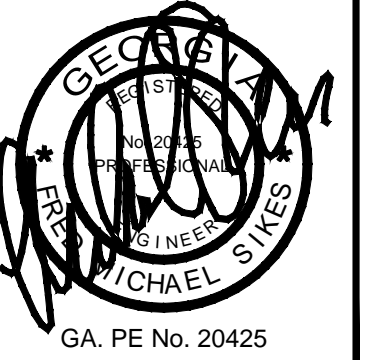
1. GENERAL: TEST PILES FURNISHED AND DRIVEN BY CONTRACTOR TO DETERMINE LENGTHS OF PILES MAY BE LOCATED, CUT OFF, AND BECOME PART OF FOUNDATION SYSTEM PROVIDED THEY CONFORM TO CONTRACT REQUIREMENTS.
2. PILE TESTING: ONE PILE LOAD TEST IS REQUIRED. THE TEST PILE SHALL BE LOADED TO 2 TIMES THE DESIGN LOAD IN ACCORDANCE WITH THE QUICK TEST PROCEDURES OF ASTM D 1143 AT THE LOCATIONS TO BE SELECTED BY THE ARCHITECT. ALL TESTS SHALL BE CONDUCTED UNDER THE SUPERVISION OF THE APPROVED SOILS ENGINEER AND DIRECTED BY A REGISTERED PROFESSIONAL GEOTECHNICAL ENGINEER. THE PILING CONTRACTOR SHALL OUTLINE IN WRITING ALL EQUIPMENT AND PROCEDURES TO BE USED AND SUBMITTED TO THE ARCHITECT FOR APPROVAL PRIOR TO TESTS. THE CONTRACTOR SHALL PROVIDE ALL NECESSARY EQUIPMENT FOR LOAD TESTS.
3. COSTS OF LOAD TESTS SHALL BE PAID FOR BY THE CONTRACTOR.
4. WHERE TEST PILES EXTEND ABOVE FINAL CUT OFF ELEVATIONS, THEY SHALL BE CASED TO THE OPERATING GROUND LEVEL TO PREVENT FRICTION EFFECTS IN THE AREA TO BE EXCAVATED.
5. MINIMUM SPACING BETWEEN TEST PILES AND REACTION PILES SHALL BE 7'-0". UNDATED REACTION PILES MAY SERVE AS JOB PILES IF IN CORRECT LOCATION AND INSTALLED IN THE SAME MANNER AS THE TEST PILES. PROVIDE SUFFICIENT TENSION REINFORCEMENT AND ACCESSORIES IN REACTION PILES TO DEVELOP REQUIRED UPLIFT LOAD WITH A SAFETY FACTOR OF 3.
6. GROUT: THE GROUT MIX SHALL BE TESTED BY MAKING ONE SET OF 2"x2" CUBES FOR EACH DAY DURING WHICH PILES ARE PLACED. A SET OF CUBES SHALL BE TESTED AT 7 DAYS AND TWO CUBES TO BE TESTED AT 28 DAYS. TEST CUBES SHALL BE MADE AND TESTED IN ACCORDANCE WITH ASTM C109, WITH THE EXCEPTION THAT THE GROUT SHOULD BE RESTRAINED FROM EXPANSION BY A TOP PLATE. TEST SAMPLES SHALL BE TAKEN AND TESTED BY AN INDEPENDENT TESTING LABORATORY APPROVED BY THE ARCHITECT AND PAID FOR BY THE CONTRACTOR.

REJECTION AND REPLACEMENT

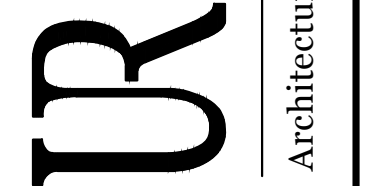
1. REJECTION: THE SOIL ENGINEER SHALL REJECT ALL PILES INSTALLED AT VARIANCE WITH SPECIFIED TOLERANCES, THAT DISPLAY HARMFUL DISTORTION OR STRUCTURAL DEFECTS, THAT WERE NOT INSTALLED IN THE REQUIRED MANNER, OR THAT FAIL TO MEET THESE SPECIFICATIONS IN ANY OTHER MANNER.
2. REPLACEMENT: ANY PILE INSTALLED IMPROPERLY OR OTHERWISE DEFECTIVE AND REJECTED, SHALL BE REMOVED AND/OR REPLACED TO THE SATISFACTION OF THE SOIL ENGINEER AND ARCHITECT AT NO ADDITIONAL COST TO THE OWNER. WHERE DEVIATIONS EXCEED THE ALLOWABLE TOLERANCES, THE ARCHITECT OR ENGINEER MAY PROVIDE WRITTEN INSTRUCTIONS TO COMPENSATE FOR RESULTING ECCENTRICITY. CORRECTIONS INCLUDING BUT NOT LIMITED TO REINFORCED TIE BEAMS, ADDITIONAL PILES OR OTHER MEANS SHALL BE THE DECISION OF THE ENGINEER AND SHALL BE CONSTRUCTED AT NO ADDITIONAL COST TO THE OWNER.

PAYMENT

1. THE BASE BID SHALL INCLUDE THE TOTAL LINEAR FEET FOR THE NUMBER AND LENGTH OF PILES SCHEDULED ON THE DRAWINGS.
2. PAYMENT SHALL BE MADE FOR THE TOTAL LINEAR FEET OF PILES ACTUALLY INSTALLED, INCLUDING PERMANENT JOB PILES AND TEST PILES. HOWEVER, NO PAYMENT WILL BE MADE FOR PILES REQUIRED TO REPLACE DEFECTIVE WORK. CONTRACT PRICE PER LINEAR FOOT INCLUDES LABOR, MATERIALS, TOOLS, EQUIPMENT, AND INCIDENTALS FOR PERFORMING WORK FOR FURNISHING AND INSTALLING PILES.
3. THE ADJUSTMENT FOR CHANGE IN TOTAL LENGTH OF PILES INSTALLED FROM TOTAL LENGTH INCLUDED IN THE BASE BID SHALL BE APPROVED BY THE ARCHITECT.
4. LENGTH OF PILES TO BE PAID FOR SHALL BE THE LENGTH BELOW THEORETICAL BUTT ELEVATION. PAYMENT FOR LINEAR FOOTAGE IN EXCESS OF THAT INDICATED ON DRAWINGS, AND CREDIT FOR LINEAR FOOTAGE LESS THAN THAT INDICATED ON DRAWINGS, SHALL BE MADE AT UNIT PRICES STATED IN THE CONTRACT, BASED ON NET ADDITION OR DEDUCTION. THE BASE BID SHALL ALSO INCLUDE THE LOAD TEST OF ONE PILE.
5. TEST PILES THAT BECOME PART OF COMPLETED FOUNDATION SYSTEM WILL BE CONSIDERED AS AN INTEGRAL PART OF WORK.
6. AS A BASIS FOR BIDS, ALL PILES SHALL BE INSTALLED TO ELEVATION SHOWN ON THE STRUCTURAL DRAWINGS.



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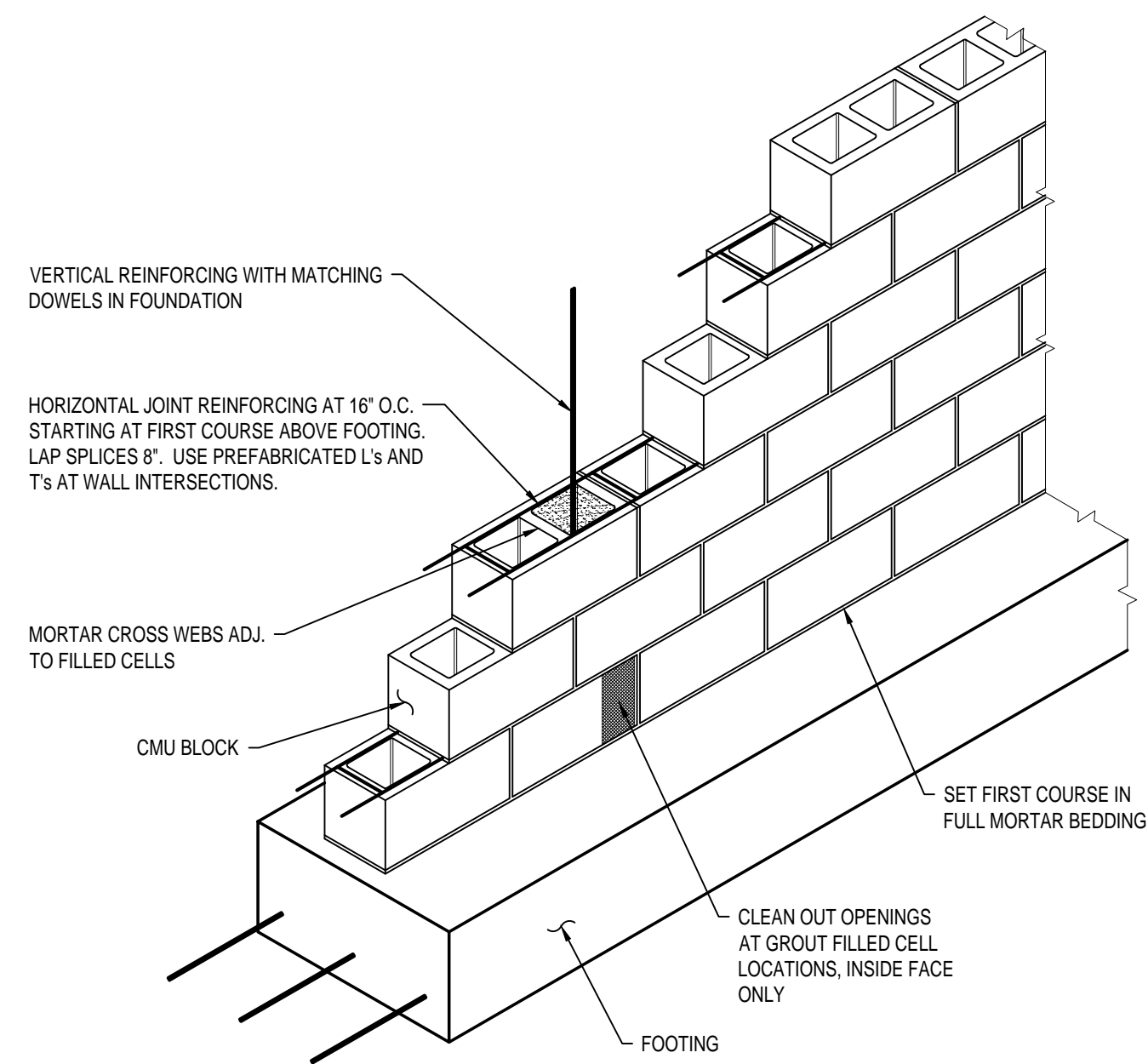
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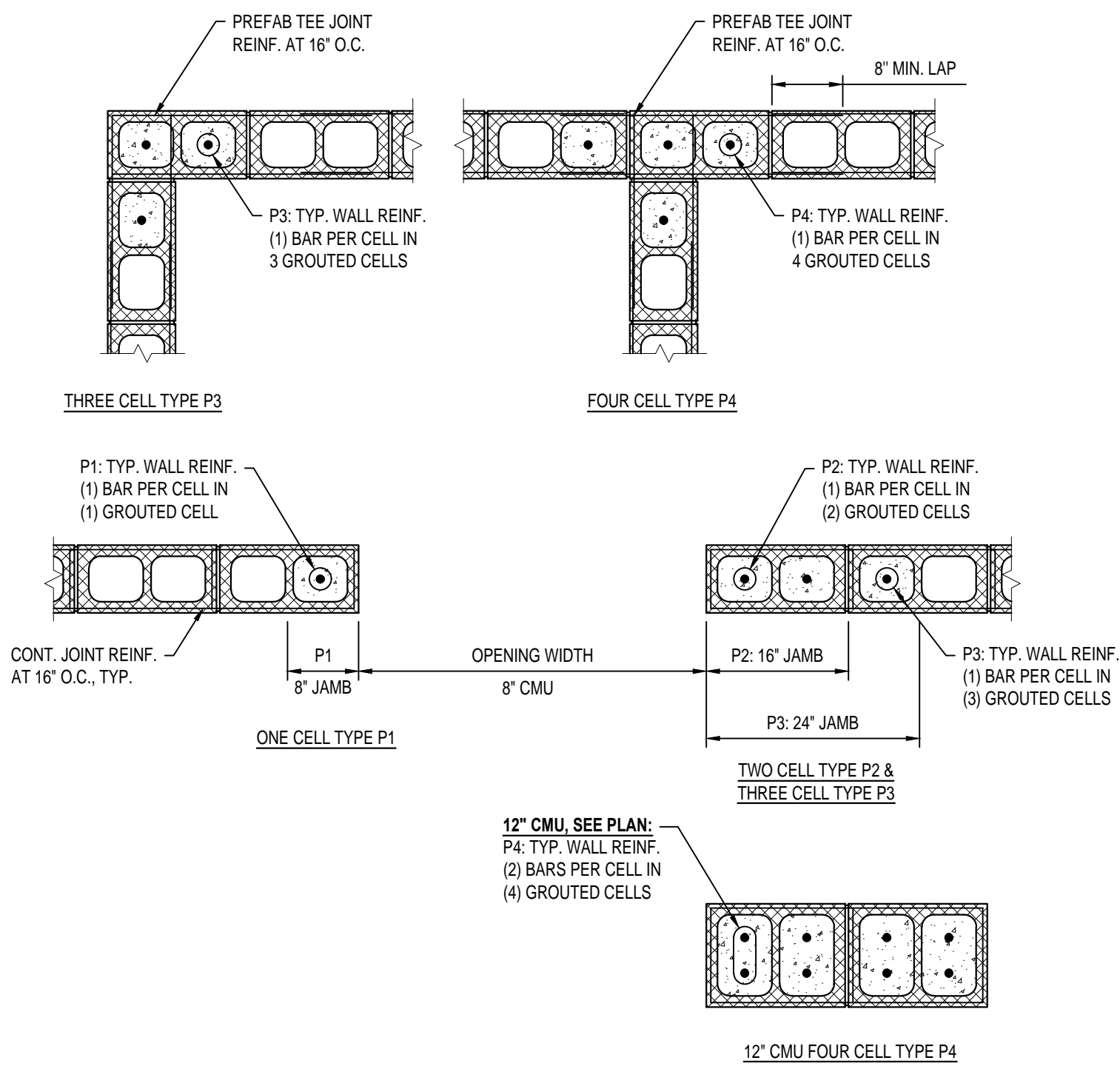
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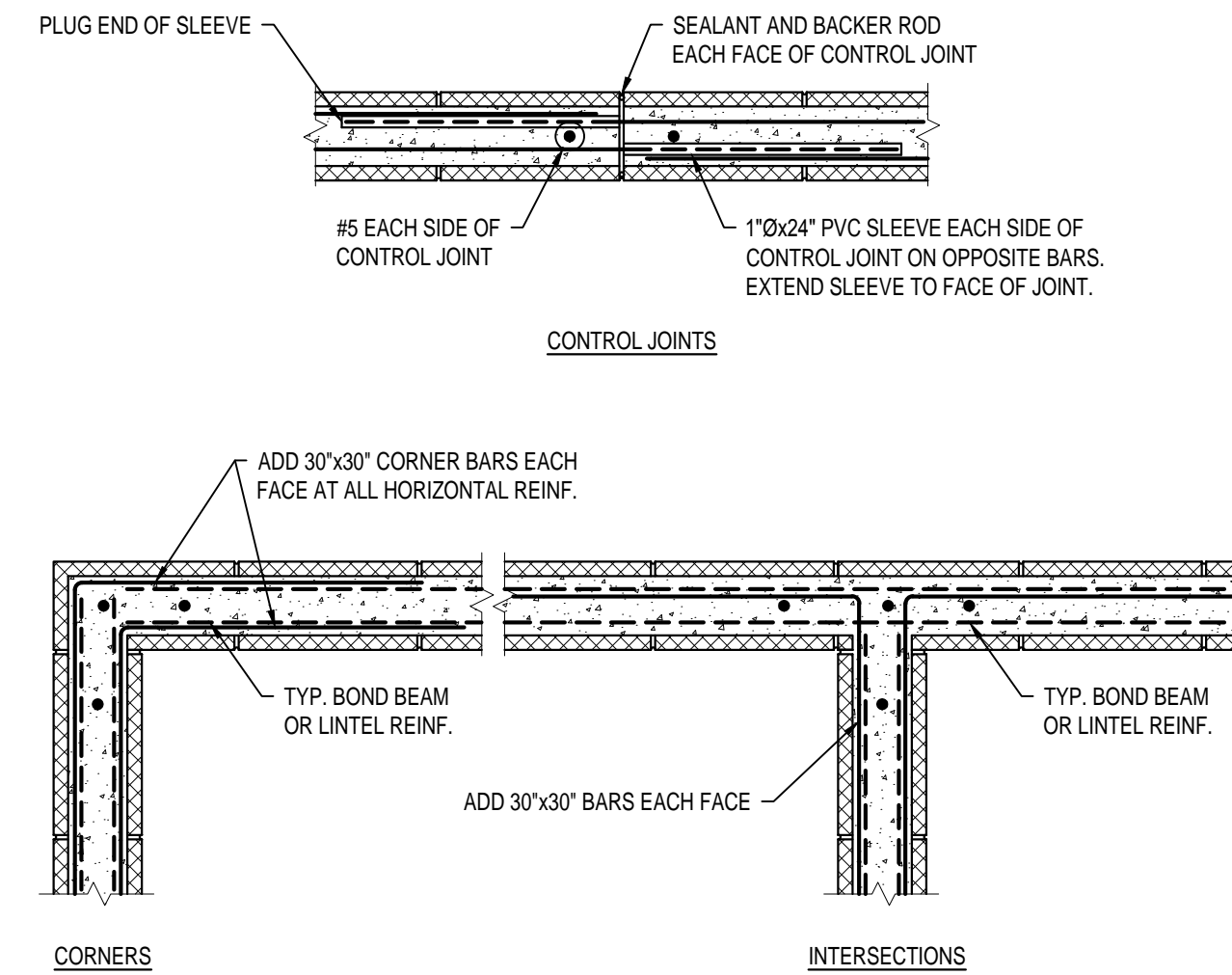
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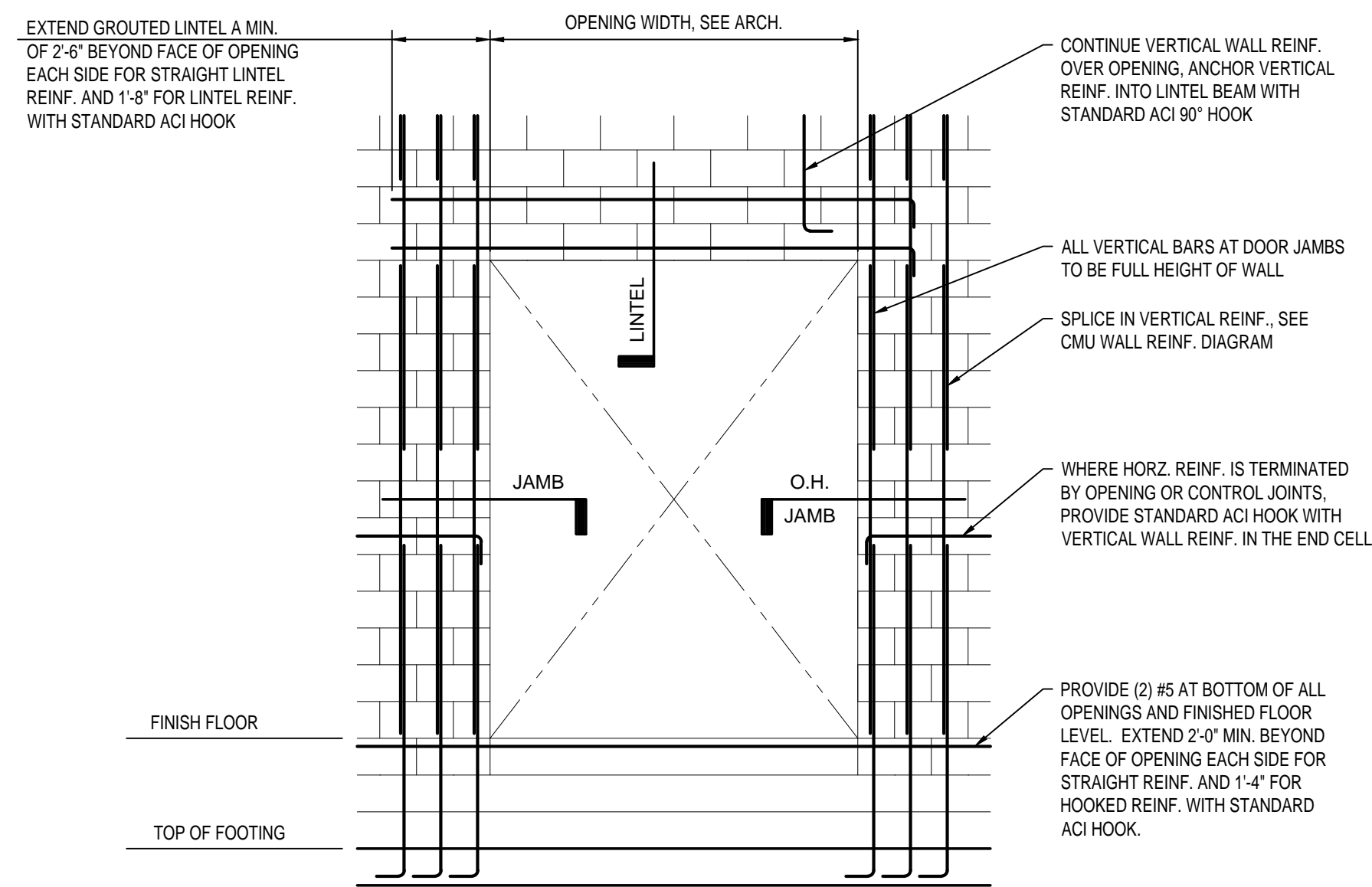
1
S0.3
TYPICAL MASONRY WALL
SCALE: 3/4"=1'-0"



2
S0.3
TYPICAL CMU CORNERS, JAMBS & INTERSECTIONS
SCALE: 3/4"=1'-0"

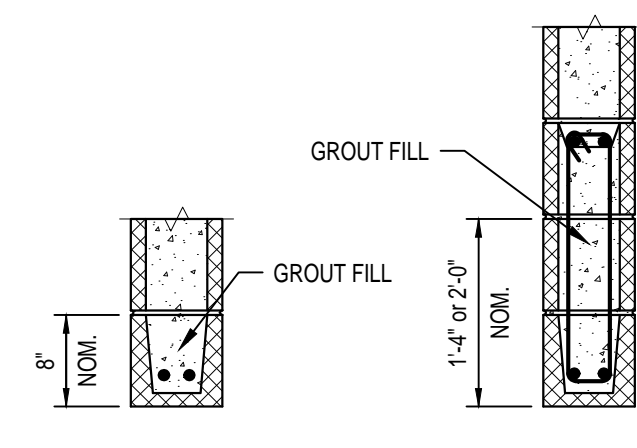


3
S0.3
TYPICAL CMU LINTELS & BOND BEAM DETAILS
SCALE: 3/4"=1'-0"



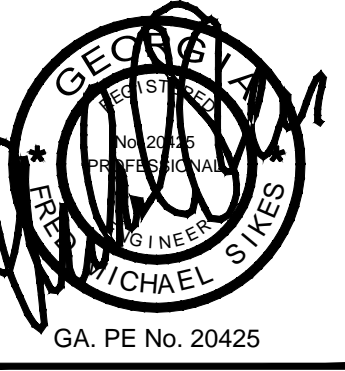
4
S0.3
TYPICAL CONCRETE MASONRY WALL OPENING DIAGRAM AND LINTEL SCHEDULE
SCALE: 3/4"=1'-0"

MAX. CLEAR SPAN	8" CMU LINTELS				REMARKS
	DEPTH	BOTTOM REINF.	TOP REINF.	TIE SPACING	
3'-4"	8"	2#5	-	-	
4'-8"	8"	2#5	-	-	
6'-0"	16"	2#6	-	-	
7'-4"	16"	2#6	2#5	#3 AT 8"	
8'-8"	16"	2#6	2#6	#3 AT 8"	
10'-0"	16"	2#8	2#6	#3 AT 6"	
11'-4"	24"	2#8	2#6	#3 AT 6"	
12'-8"	24"	2#8	2#7	#3 AT 6"	
14'-0"	24"	2#8	2#8	#3 AT 6"	



MASONRY LINTEL NOTES:

- MINIMUM MASONRY STRENGTH SHALL BE $f_m = 1500$ PSI.
- NOMINAL MASONRY DIMENSIONS SHOWN RATHER THAN ACTUAL DIMENSIONS.
- GROUT FILL SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI AT 28 DAYS OR MEET ASTM C476.
- END BEARING SHALL BE 6" MINIMUM FOR CLEAR SPANS OF 6'-0" OR LESS AND 16" FOR CLEAR SPANS EXCEEDING 6'-0".
- TOP AND BOTTOM REINFORCEMENT SHALL EXTEND 8" MINIMUM OVER SUPPORT AT EACH END.
- VERTICAL WALL REINFORCING AT BEARING SHALL EXTEND THROUGH LINTEL.
- WHERE LINTEL BEARING IS AT A JAMB ADJACENT TO A VERTICAL CONTROL JOINT, OFFSET CONTROL JOINT OVER OPENING TO LIMIT OF BEARING. DISCONTINUE VERTICAL REINFORCING AT JAMB BELOW AND ADD 3/8" PEJ BOND BREAK AT BEARING.



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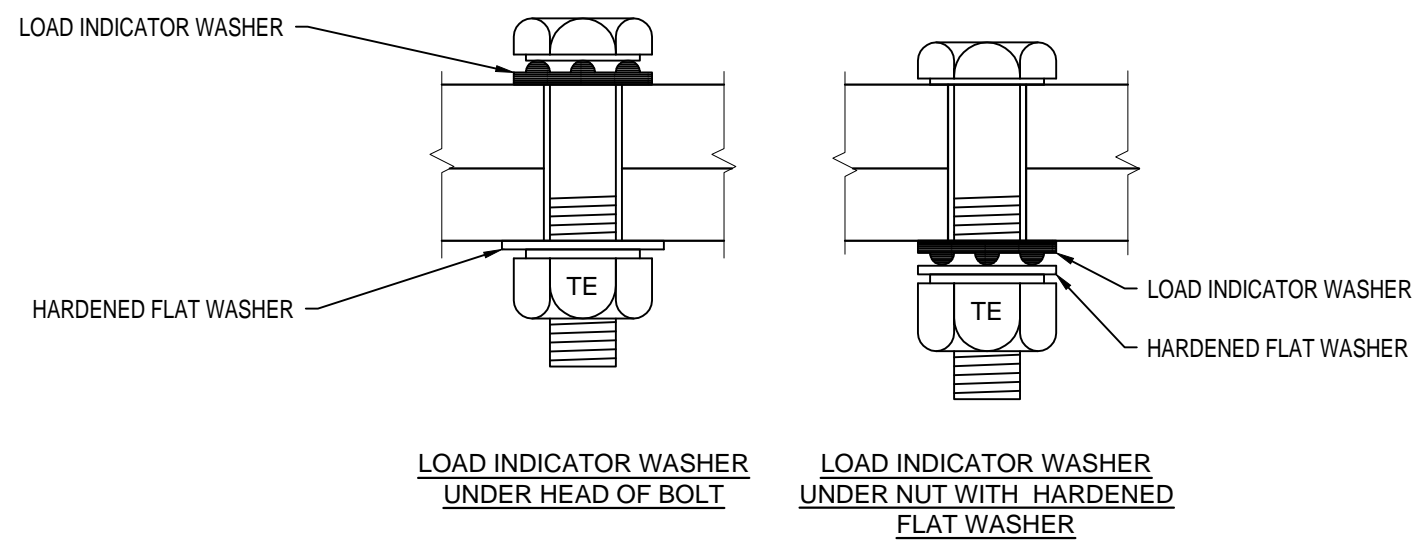
S0.3

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TABLE OF LOAD INDICATOR GAPS REQUIRED TO YIELD BOLT MINIMUM DESIGN TENSION		
LOAD INDICATOR FITTING	A325	A490
UNDER BOLT HEAD	0.015 IN.	0.015 IN.
UNDER NUT WITH NUT FACE WASHERS	0.005 IN.	0.005 IN.

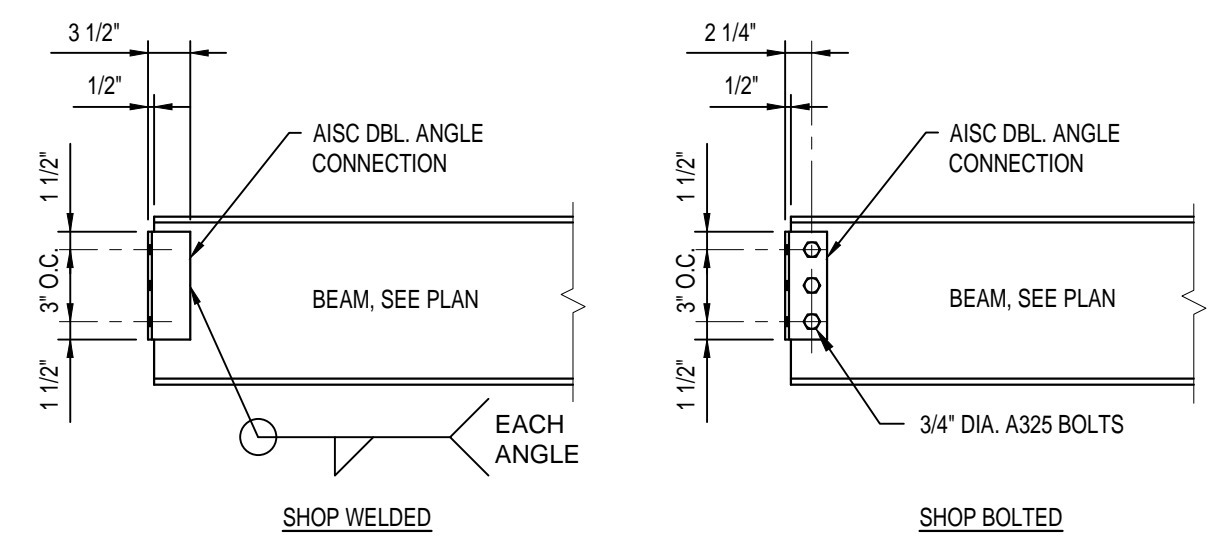
WITH GAPS INDICATED ABOVE, REQUIRED MINIMUM BOLT TENSIONS WILL BE DEVELOPED AS INDICATED IN TABLE.

MINIMUM FASTENER TENSION IN THOUSANDS OF POUNDS (KIPS)		
BOLT DIA. (IN.)	A325	A490
1/2	12	15
5/8	19	24
3/4	28	35
7/8	39	49
1	51	64
1 1/8	56	80
1 1/4	71	102



1
S0.4 **INSTALLATION OF LOAD INDICATOR WASHERS**

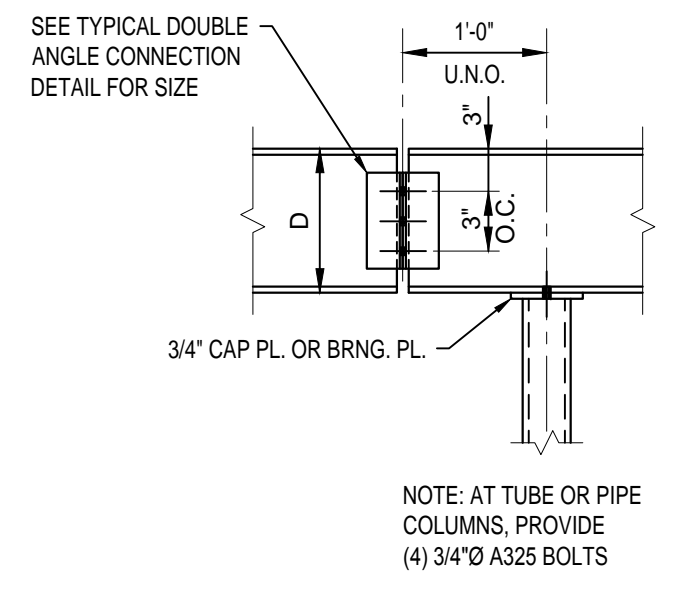
- SEE MANUFACTURER'S LITERATURE FOR INSTRUCTIONS AND SPECIFICATIONS.
- INSTALLATIONS OTHER THAN THOSE SHOWN ABOVE SHALL BE APPROVED IN WRITING BY ENGINEER PRIOR TO FABRICATION OR INSTALLATION.
- "TE" INDICATES TURNED ELEMENT.



CONNECTION SCHEDULE		
BEAM SIZE	# ROWS BOLTS	JLMIN. THICKNESS
W8, W10	2 ROWS	1/4"
W12, W14	3 ROWS	1/4"
W16, W18	4 ROWS	5/16"
W12, W24	5 ROWS	5/16"
W27, W30, W36	6 ROWS	5/16"

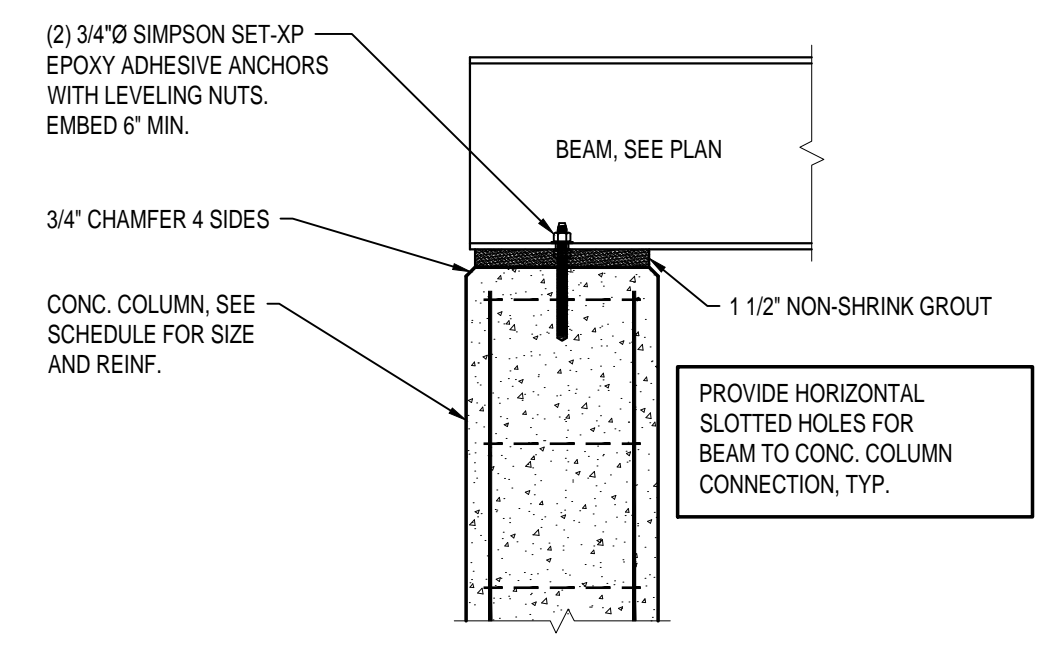
- NOTES:
- ALL BEAM SIZES MAY NOT BE USED ON THIS PROJECT.
 - WHEN CONNECTING BEAMS OF DIFFERENT DEPTHS, USE SMALLER BEAM SIZE TO DETERMINE # OF ROWS OF BOLTS.
 - FOR CONNECTIONS TO BE DESIGNED BY SUPPLIER, THE ANGLE THICKNESS AND NUMBER OF ROWS OF BOLTS SHOWN IN THIS DETAIL SHALL BE THE MINIMUM ACCEPTABLE.
 - FOR CONNECTIONS NOT DETAILED, SUPPLIER SHALL DESIGN THE CONNECTION FOR THE REACTIONS SHOWN ON THE PLANS. IF NO REACTION IS PROVIDED, DESIGN FOR 1/2 THE MAXIMUM UNIFORM LOAD PER PART 2 OF THE AISC MANUAL FOR STEEL CONSTRUCTION (ASD).
 - CONNECTIONS MAY BE SHOP WELDED OR SHOP BOLTED TO BEAM AT SUPPLIER'S OPTION.
 - DOUBLE ANGLE CONNECTIONS MUST COMPLY WITH OSHA REGULATIONS.

2
S0.4 **TYPICAL DOUBLE ANGLE CONNECTIONS**

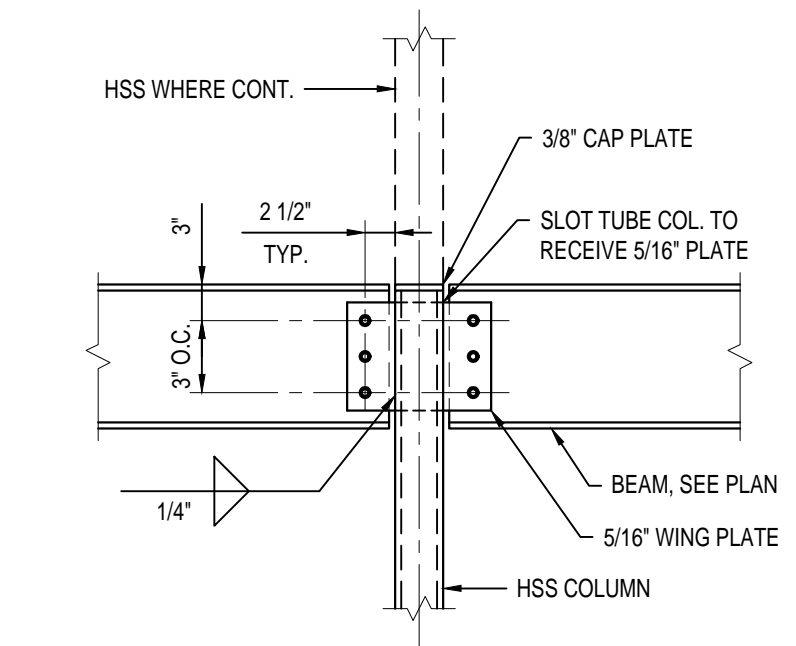


5
S0.4 **BEAM SPLICE DETAIL**

4
S0.4 **BEAM CONNECTION TO CONCRETE COLUMN**

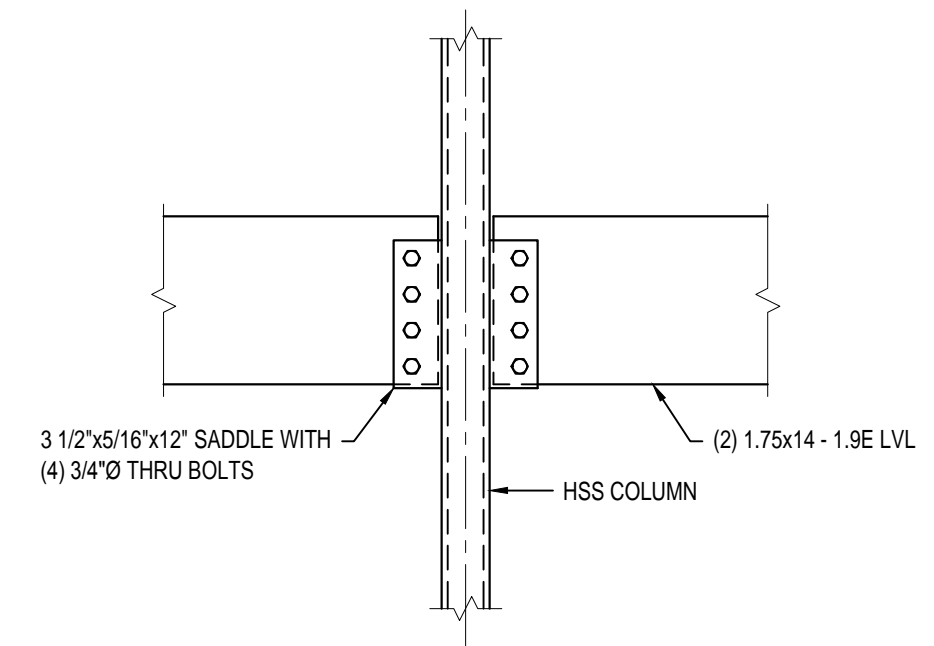


6
S0.4 **TYPICAL BEAM TO HSS COLUMN CONNECTION**

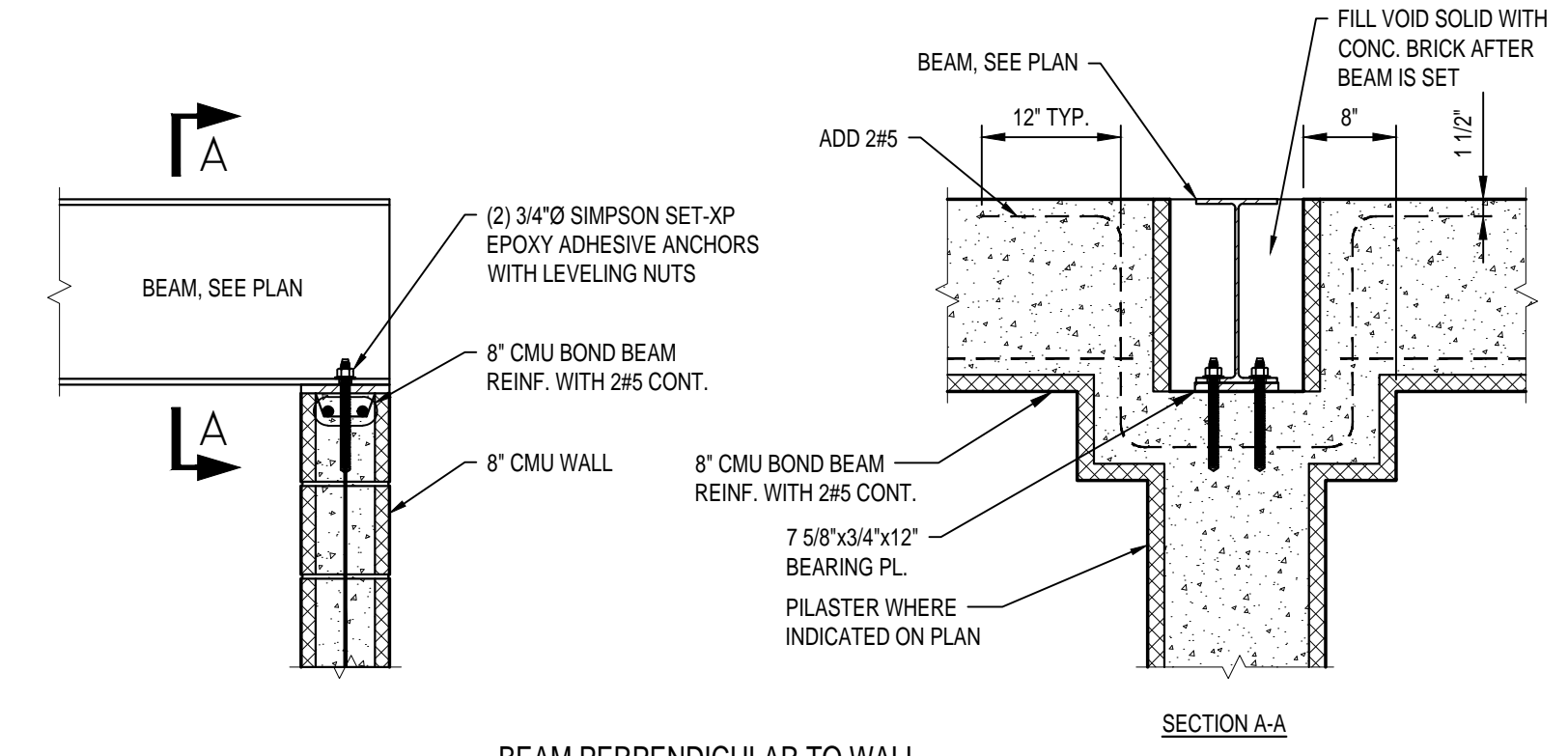


6
S0.4 **TYPICAL BEAM TO HSS COLUMN CONNECTION**
SCALE: 3/4"=1'-0"

7
S0.4 **LVL BEAM TO HSS COLUMN CONNECTION**
SCALE: 3/4"=1'-0"



7
S0.4 **LVL BEAM TO HSS COLUMN CONNECTION**
SCALE: 3/4"=1'-0"

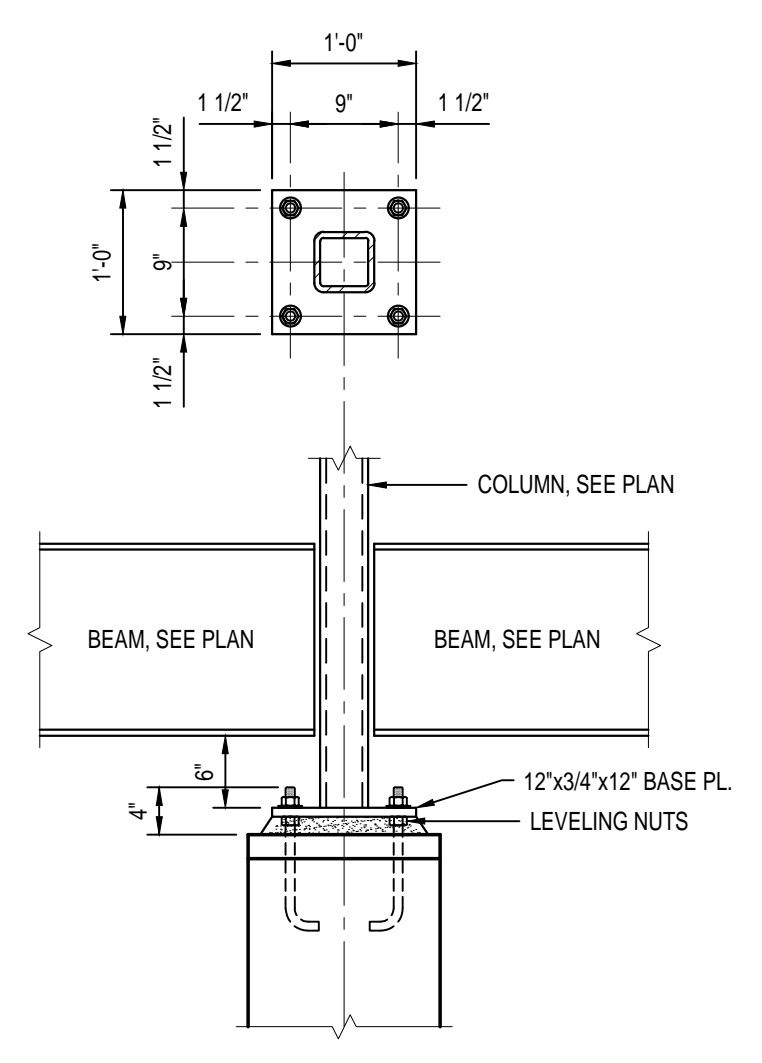


D	L	BEARING PLATE
4"-8"	8"	7 1/2"x12"x7 1/2"
9"-12"	12"	7 1/2"x3/4"x1'-0"
14"-16"	16"	7 1/2"x3/4"x1'-2"

L INDICATES LENGTH OF BEARING PARALLEL TO WALL. BEAMS PERPENDICULAR TO WALL SHALL BEAR FULL WIDTH LESS 2", IF MASONRY EXPOSED ON OPPOSITE FACE TO FULL WIDTH, IF BRICK VENEERED ON OPPOSITE FACE.

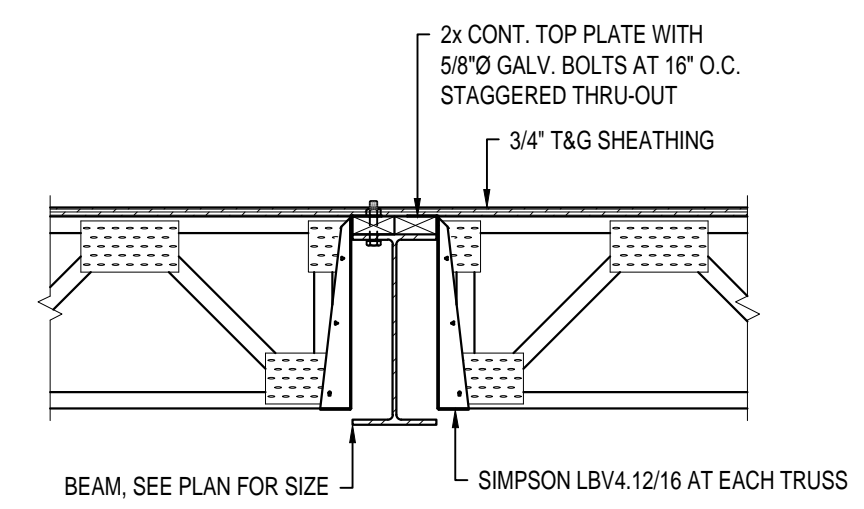
3
S0.4 **STEEL BEAM BEARING ON CMU WALL**

- BEARING NOTES
- PROVIDE HORIZONTAL SLOTTED HOLES IN BEAMS.
 - PROVIDE ONE LAYER OF 30 LB. BLDG. FELT BETWEEN BEAM AND BEARING PLATE.
 - NUTS SHALL BE HAND TIGHT ONLY.

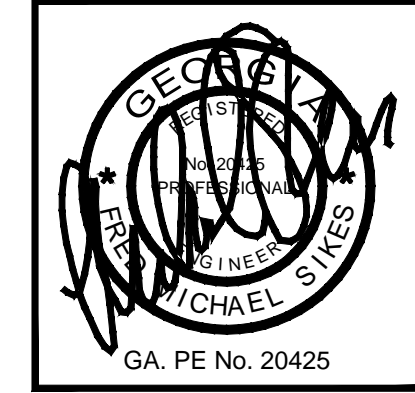


8
S0.4 **COLUMN BASE PLATE DETAIL**
SCALE: 3/4"=1'-0"

- BASE PLATE NOTES:
- ALL BASE PLATES SHALL BE ASTM A36, Fy = 36 KSI MATERIAL.
 - ALL ANCHOR RODS SHALL BE 3/4" Ø x 12" + 2" HKD. ASTM F1554 WITH HEAVY HEX NUTS, FLAT WASHERS AND LEVELING NUTS.
 - PROVIDE NON-METALLIC, NON-SHRINK GROUT (Fc = 5,000 PSI MIN.) BELOW ALL COLUMN BASE PLATES IMMEDIATELY AFTER STEEL FRAMING HAS BEEN SQUARED AND PLUMBED.



9
S0.4 **TYPICAL TRUSS TO BEAM CONNECTION**
SCALE: 3/4"=1'-0"



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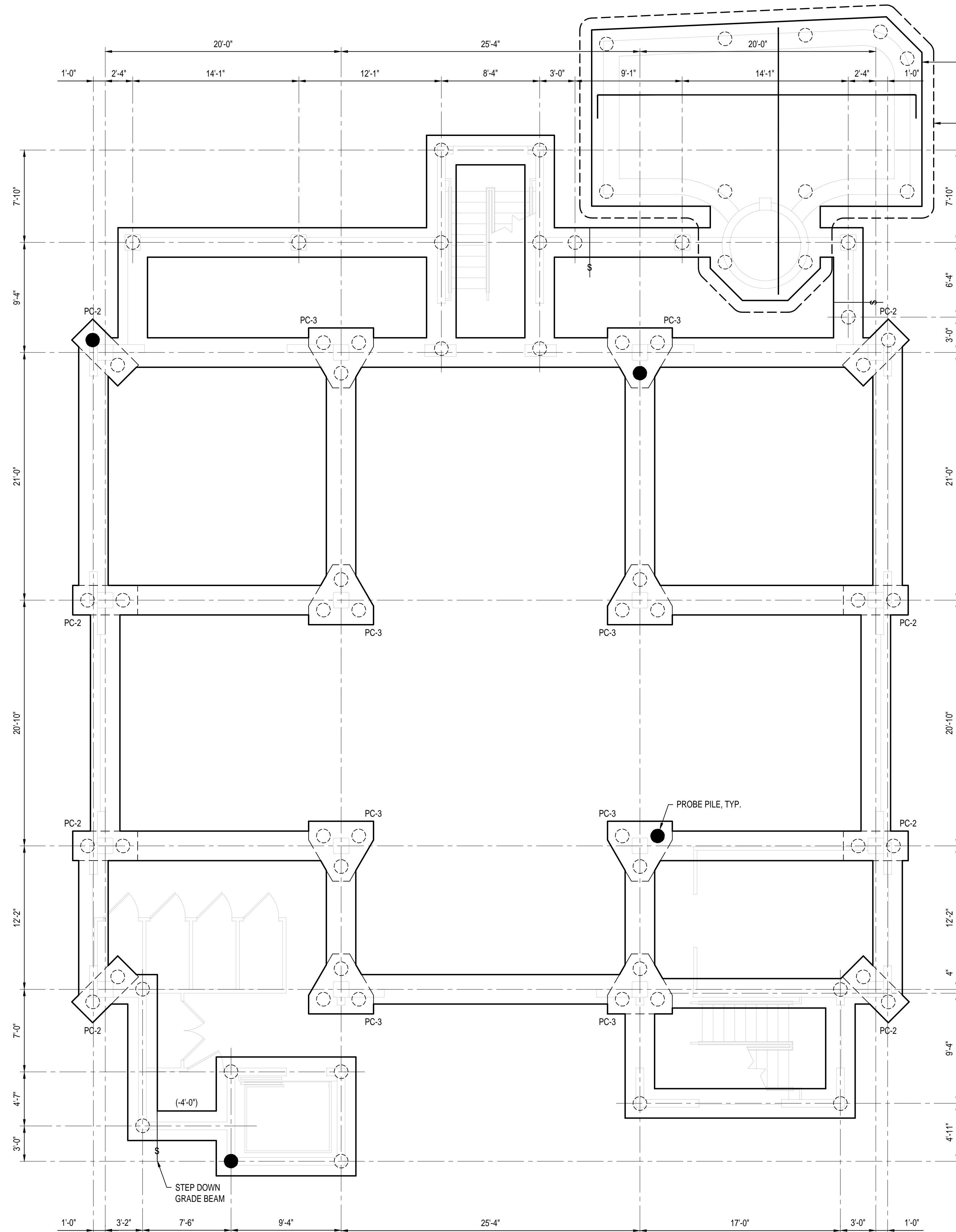
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S0.4

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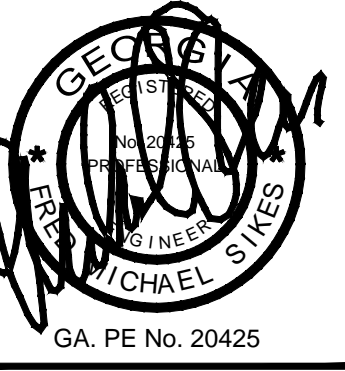


24" POOL SLAB REINF. WITH #8 AT 8" O.C. EACH WAY TOP & BOTTOM. HOOK TOP BARS 90° AT ENDS.

CONTRACTOR SHALL PROVIDE DIMENSIONED POOL LAYOUT DRAWINGS TO ENGINEER TO DETERMINE FINAL PILE LAYOUT.

- PILE LAYOUT PLAN NOTES:**
1. AUGERED CAST-IN-PLACE CONCRETE PILES SHALL DEVELOP A SAFE WORKING CAPACITY OF 30 TONS (60,000 LBS.) PER PILE WITH A SAFETY FACTOR OF 2.
 2. ALL PILES SHALL BE MINIMUM 14"Ø AND PLACED TO A MINIMUM DEPTH OF 40' BELOW EXISTING GROUND SURFACE. PILES SHALL BE REINFORCED WITH ONE FULL LENGTH #8 BAR AND A REINFORCING CAGE OF #4s WITH HOOPS AT 10" O.C. IN THE UPPER 20'. REINFORCING SHALL BE INSTALLED AFTER GROUT FILL.
 2. TOP OF PILE CAPS SHALL BE (-2'-0") BELOW TOP OF PARKING SLAB UNLESS NOTED OTHERWISE ON PLAN.
 3. VERIFY ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS.

1 PILE LAYOUT PLAN
SCALE: 3/16"=1'-0"



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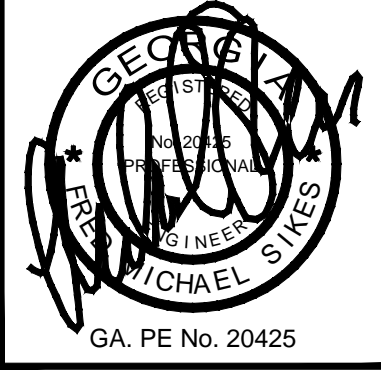
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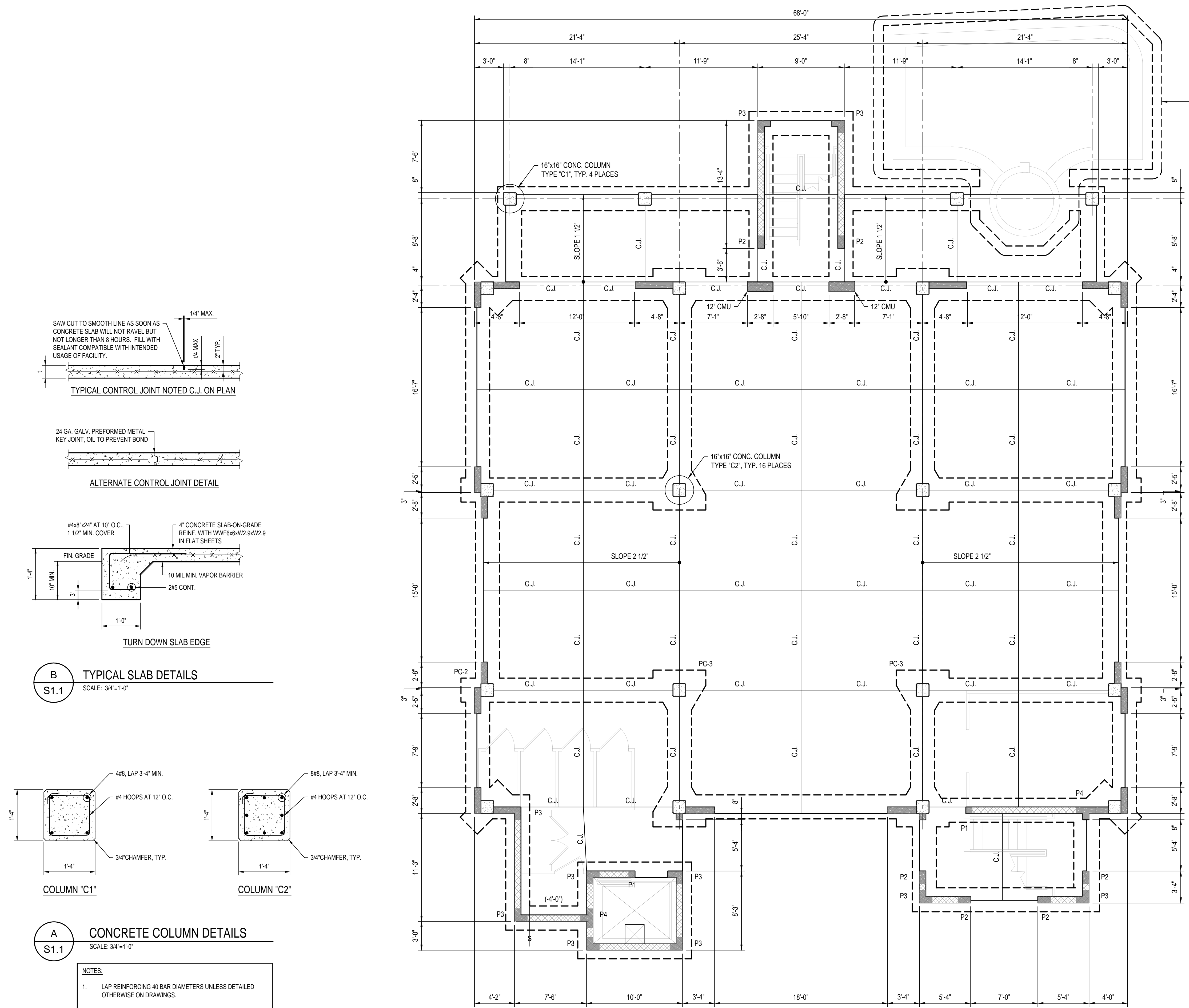
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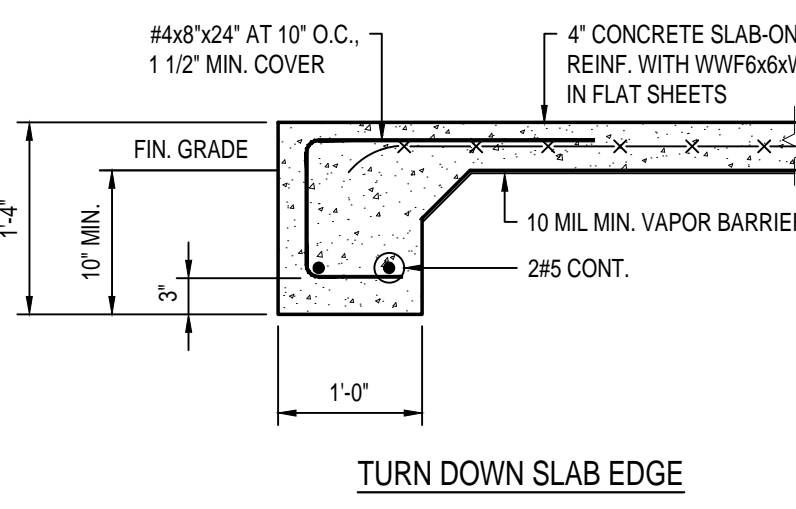
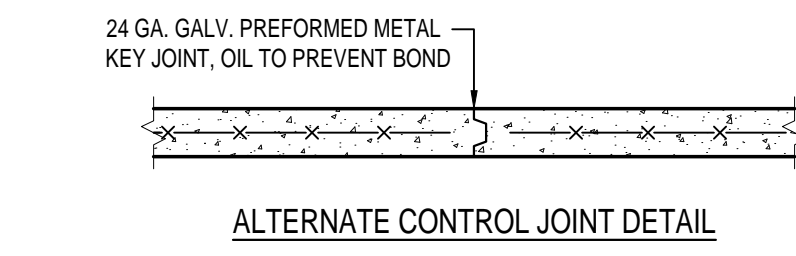
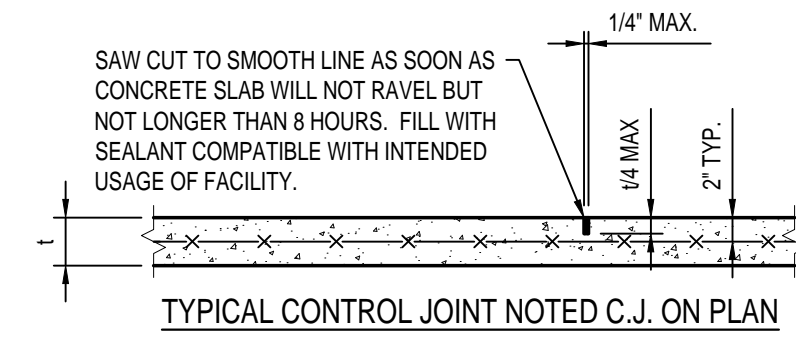
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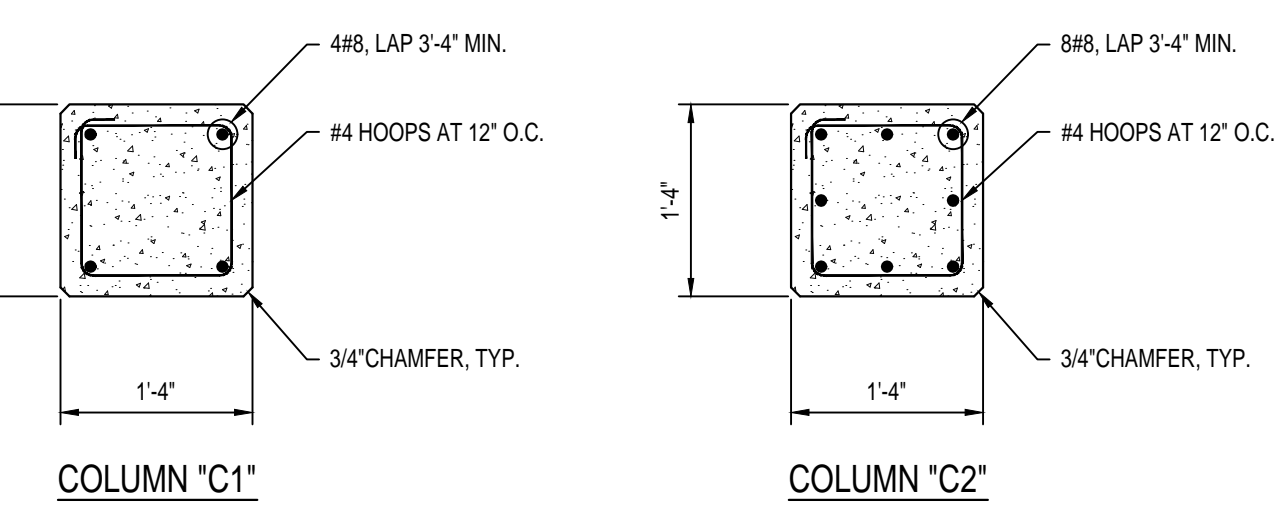
S1.1



CONTRACTOR SHALL PROVIDE DIMENSIONED POOL LAYOUT DRAWINGS TO ENGINEER TO DETERMINE FINAL PILE LAYOUT.



B TYPICAL SLAB DETAILS
 SCALE: 3/4"=1'-0"



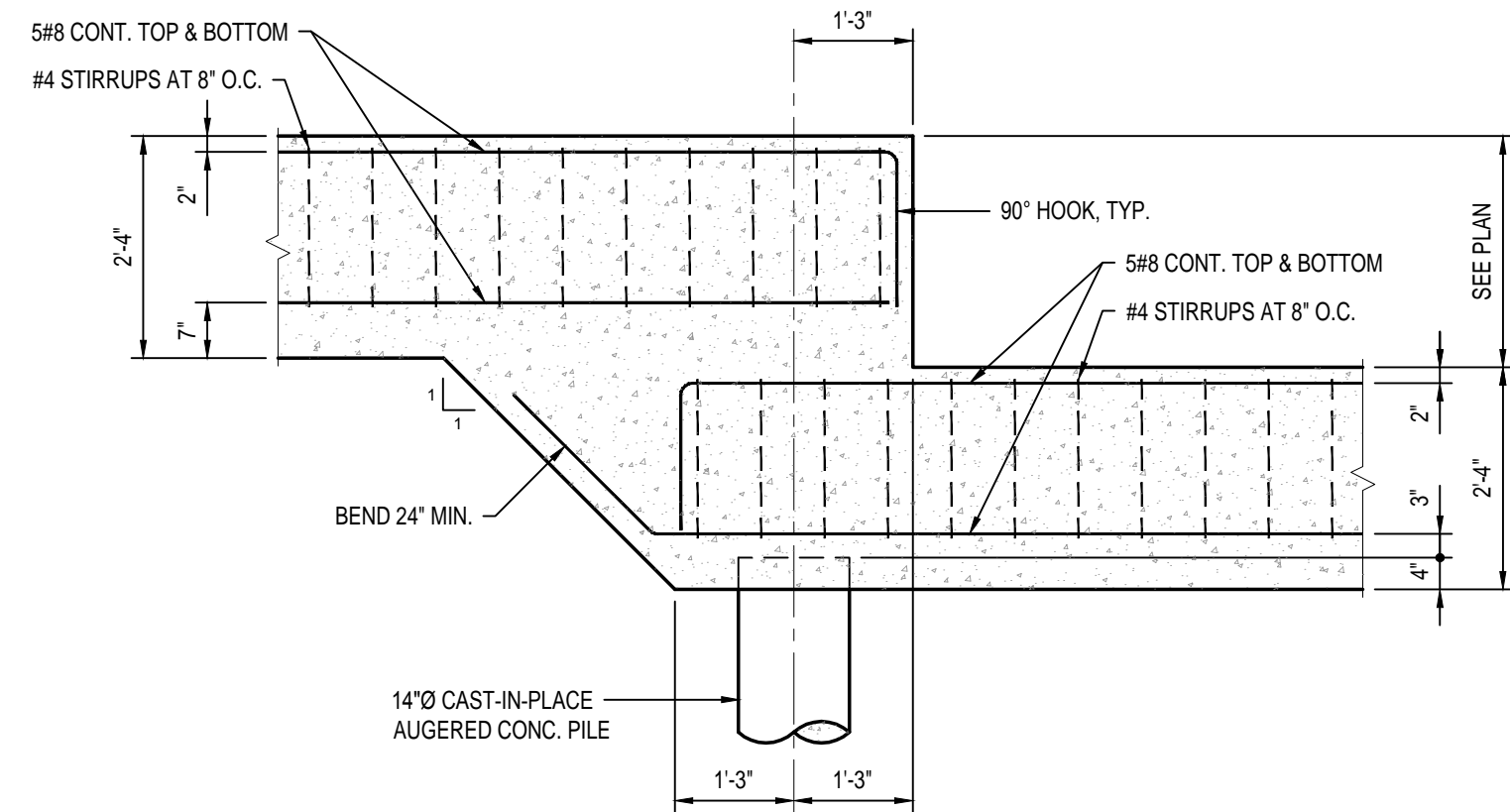
A CONCRETE COLUMN DETAILS
 SCALE: 3/4"=1'-0"

- NOTES:**
- LAP REINFORCING 40 BAR DIAMETERS UNLESS DETAILED OTHERWISE ON DRAWINGS.
 - PROVIDE 3/4" CHAMFER ALONG ALL EXPOSED EDGES.
 - STRIP FORMS 48 HOURS AFTER PLACING CONCRETE AND IMMEDIATELY REPAIR ALL HONEYCOMB AREAS AND SURFACE DEFECTS.

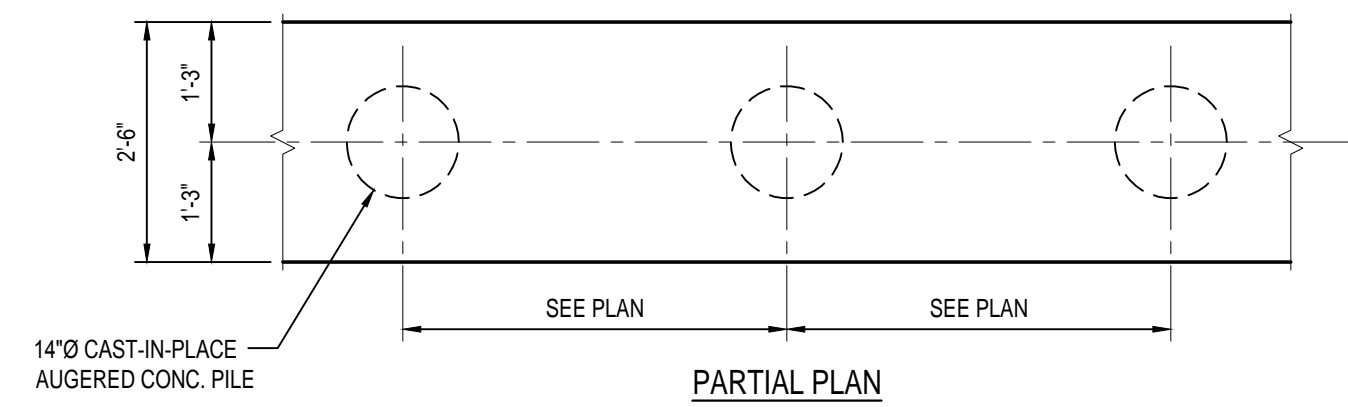
1 FOUNDATION PLAN
 SCALE: 3/16"=1'-0"

- FOUNDATION PLAN NOTES:**
- FINISH SLAB ELEVATION 8.00 MSL SHALL BE VERIFIED WITH CIVIL AND ARCH. DWGS.
 - CONCRETE SLAB -ON-GRADE SHALL BE MINIMUM 4" THICK AND REINFORCED WITH WWF 6x6xW2.9xW2.9 IN FLAT SHEETS OR FIBERMESH BATCHED AT 3.0 LBS. PER CUBIC YARD.
 - C.J. INDICATES 1" DEEP SAW-CUT CONTROL JOINT IN SLAB. SEE TYPICAL FLOOR SLAB DETAILS FOR ADDITIONAL INFORMATION.
 - PROVIDE MINIMUM 24"x24"x24" SUMP IN ELEVATOR PIT SLAB. COORDINATE SIZE AND LOCATION WITH MECHANICAL AND ELEVATOR EQUIPMENT DRAWINGS.
 - P1, P2, ETC. INDICATES REINFORCED CMU CORNERS AND INTERSECTIONS. SEE DETAILS FOR ADDITIONAL INFORMATION.
 - ALL CMU ELEMENTS DIMENSIONED 4'-0" OR GREATER SHALL BE REINFORCED WITH 1#6 AT 16" O.C.. ALL CMU ELEMENTS DIMENSIONED LESS THAN 4'-0" SHALL BE REINFORCED WITH 1#6 AT 8" O.C.. ALL CMU CELLS SHALL BE FILLED SOLID WITH CONCRETE.
 - VERIFY ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS.

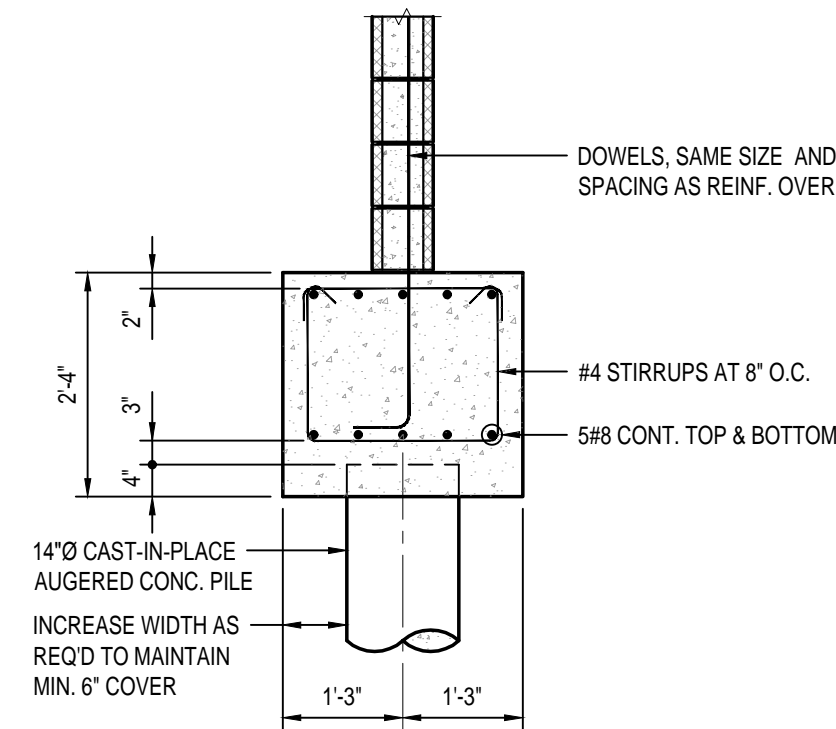
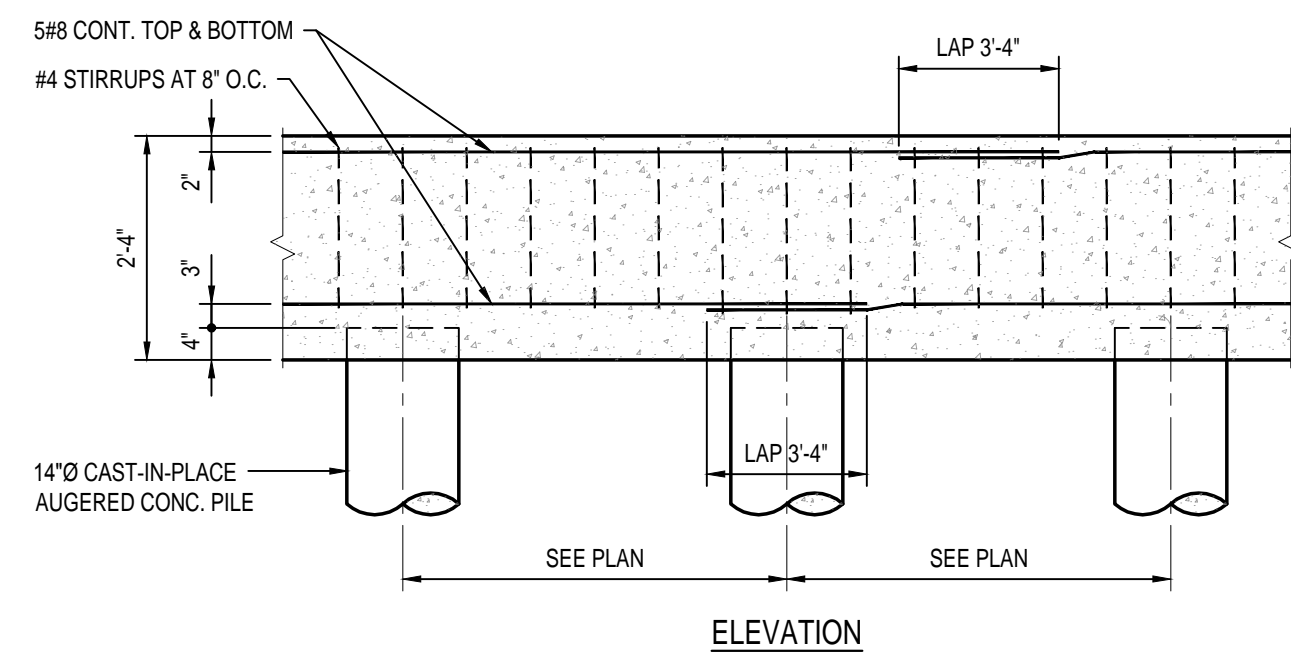
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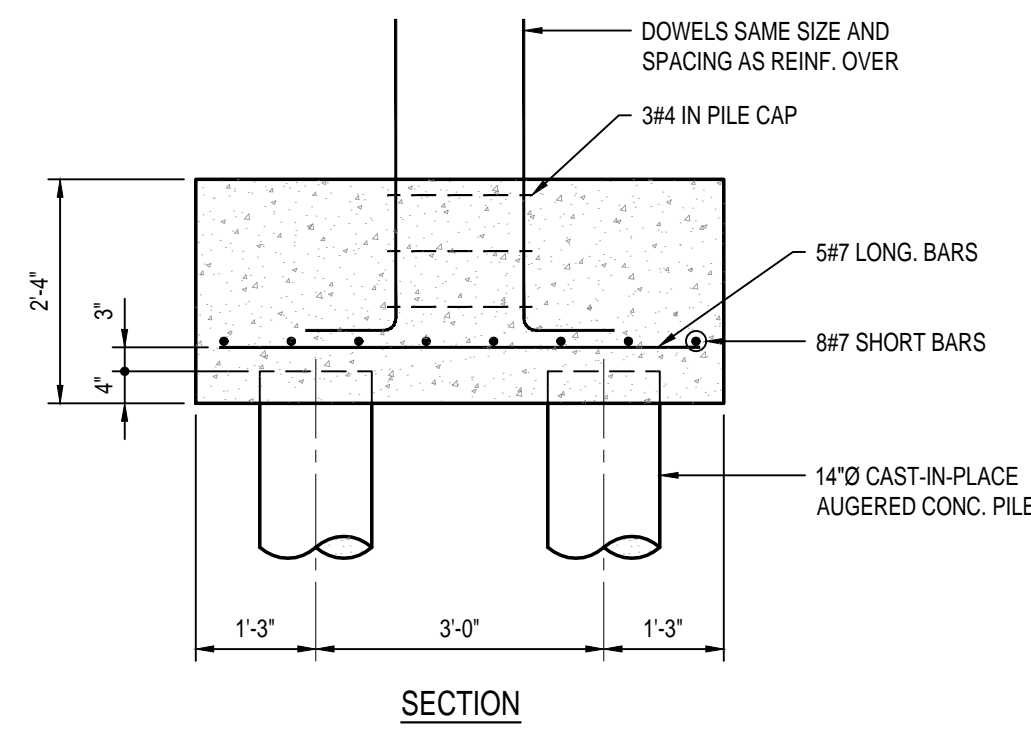
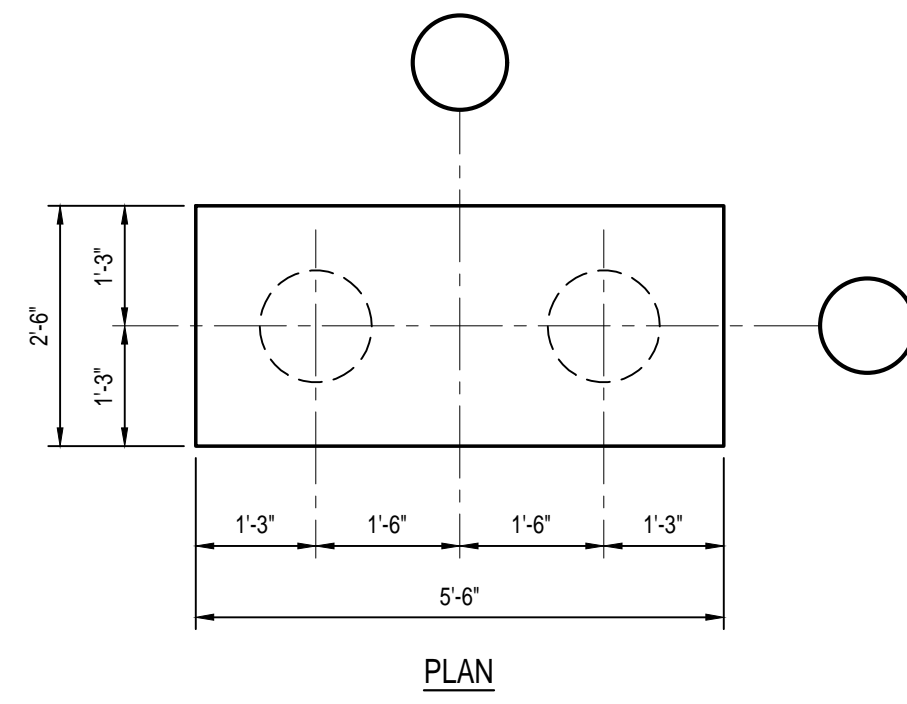
B
STEP IN GRADE BEAM DETAIL
SCALE: 1/2"=1'-0"



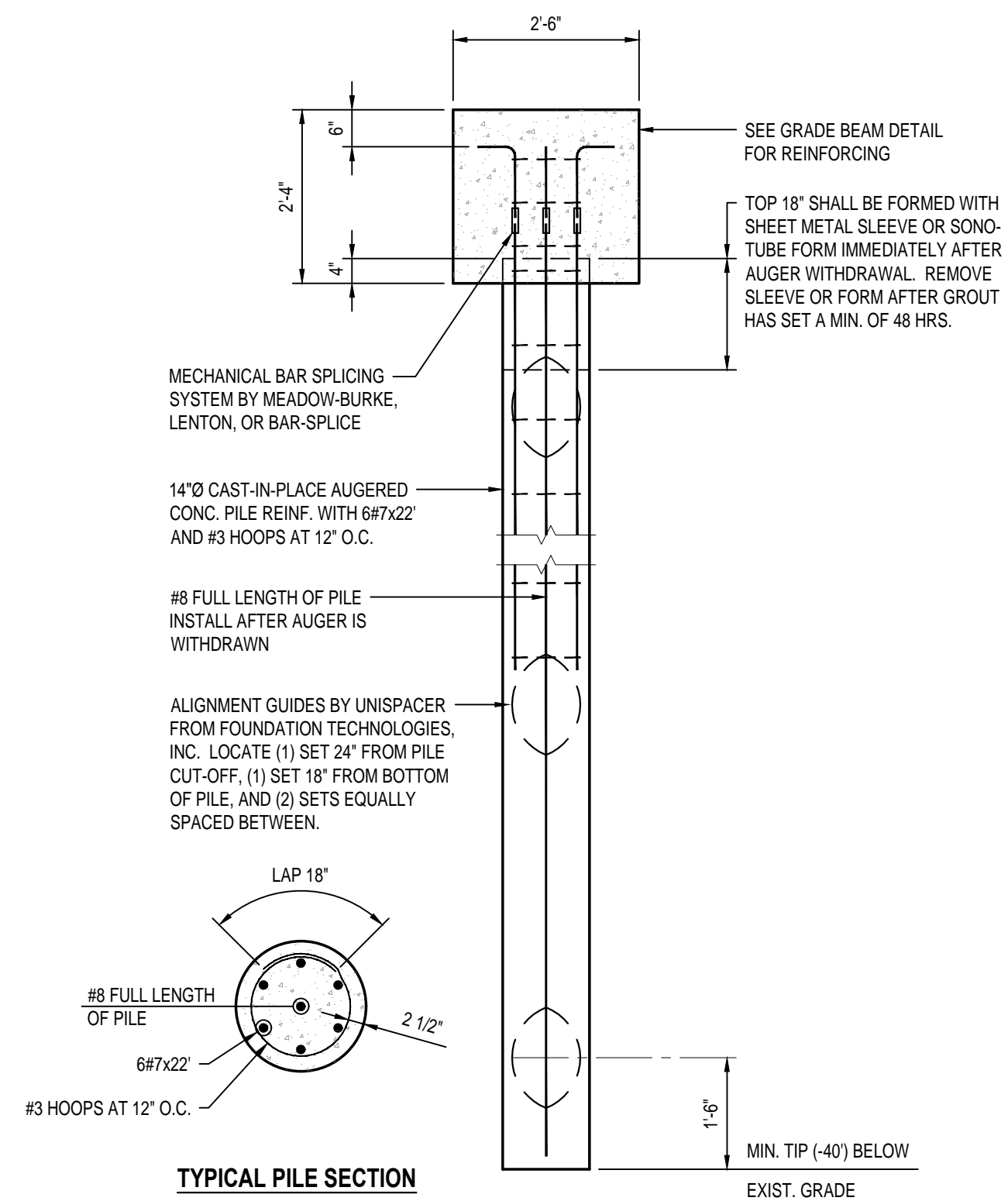
A
TYPICAL GRADE BEAM DETAIL
SCALE: 1/2"=1'-0"



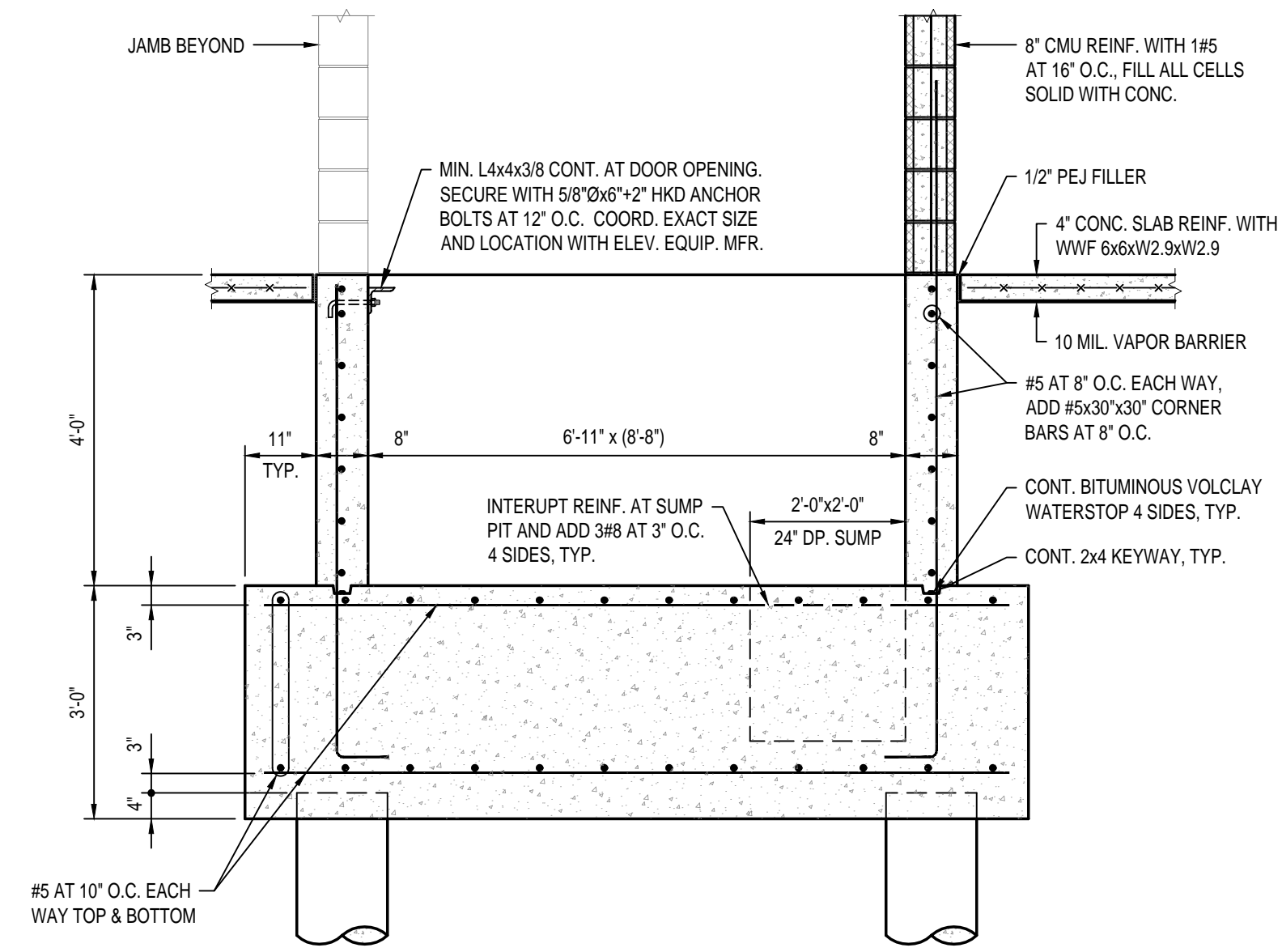
- GRADE BEAM NOTES**
1. LAPS IN TOP BARS SHALL BE 3'-4" MINIMUM AND SHALL OCCUR AT MID-SPAN BETWEEN PILES. LAPS IN BOTTOM BARS SHALL BE 3'-4" MINIMUM AND SHALL OCCUR OVER TOP OF PILES ONLY.
 2. CONSTRUCTION JOINTS ARE PERMITTED ONLY WITHIN MIDDLE THIRD OF SPAN.
 3. PROVIDE ACI STANDARD 90° HOOK IN TOP BARS AT EACH END OF GRADE BEAMS.
 4. GRADE BEAM SIDES SHALL BE BOARD FORMED.



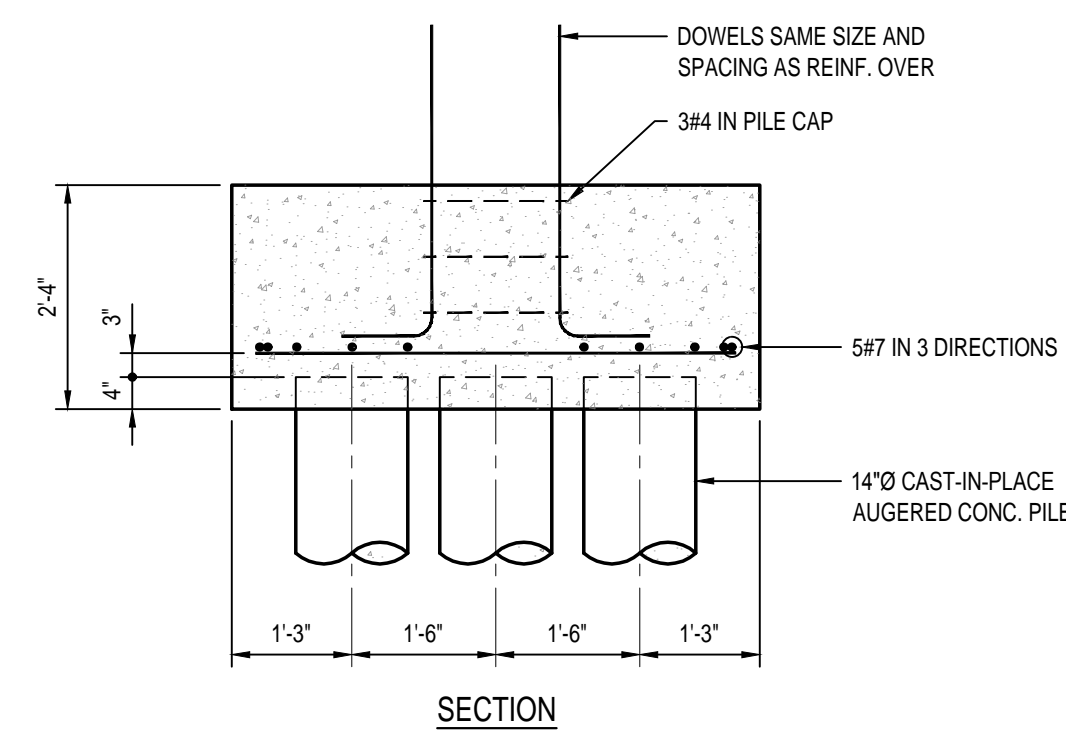
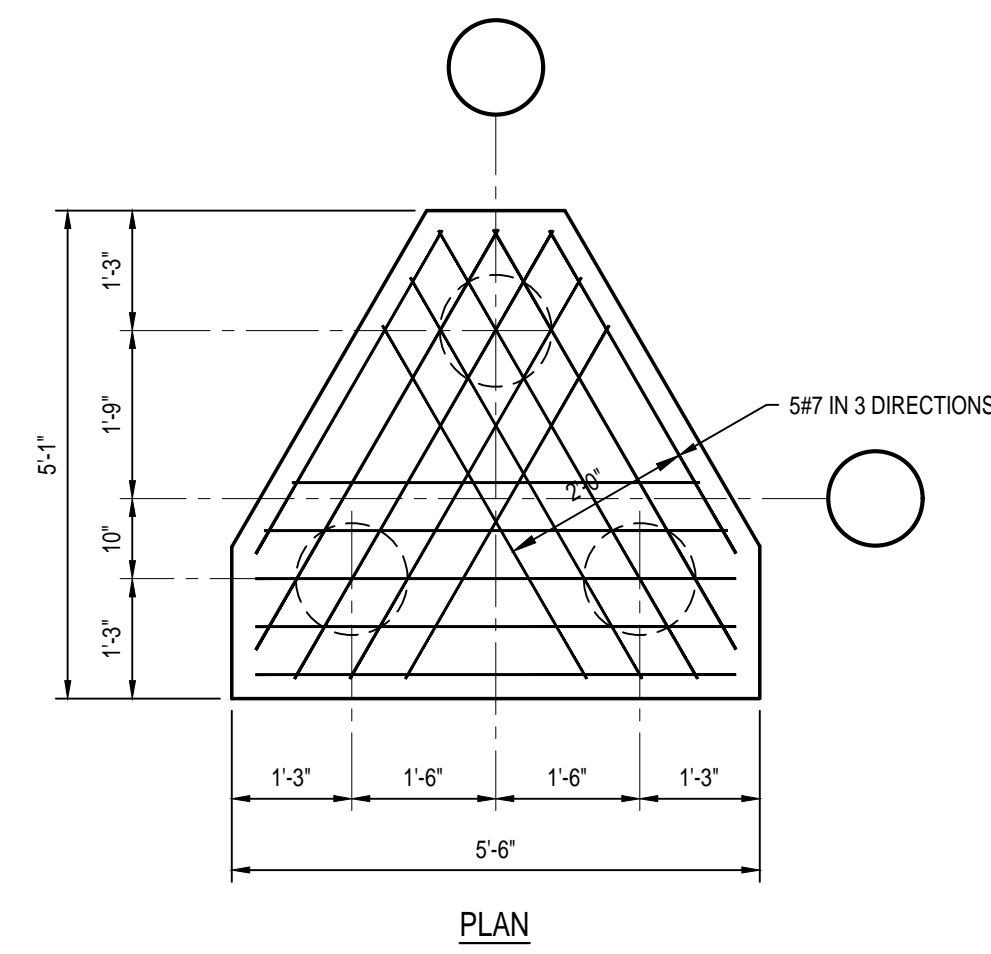
D
PILE CAP TYPE PC-2
SCALE: 1/2"=1'-0"



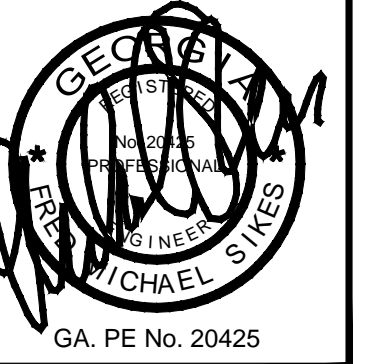
C
TYPICAL PILE ELEVATION
SCALE: 1/2"=1'-0"



F
TYPICAL ELEVATOR PIT
SCALE: 1/2"=1'-0"



E
PILE CAP TYPE PC-3
SCALE: 1/2"=1'-0"



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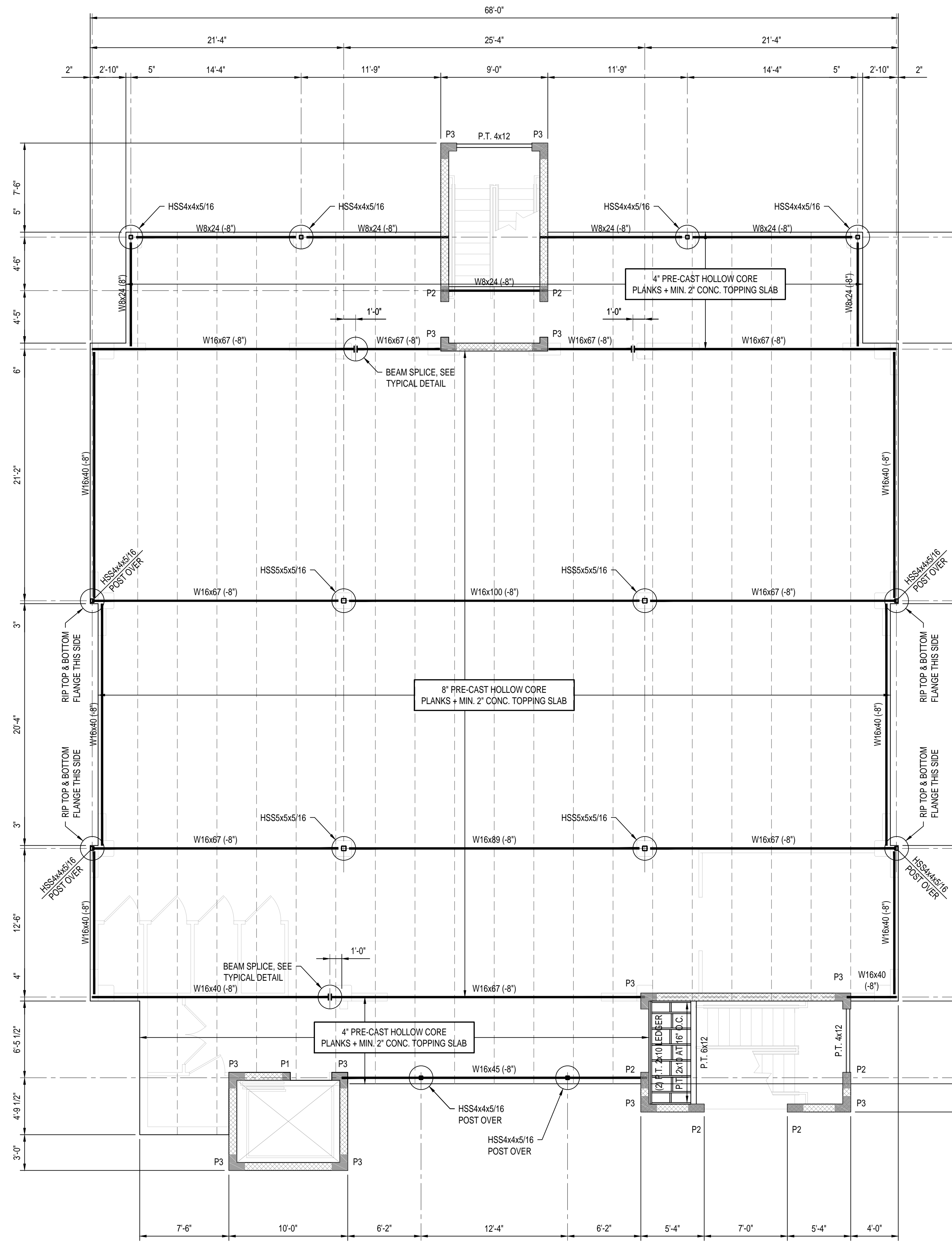
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S1.2

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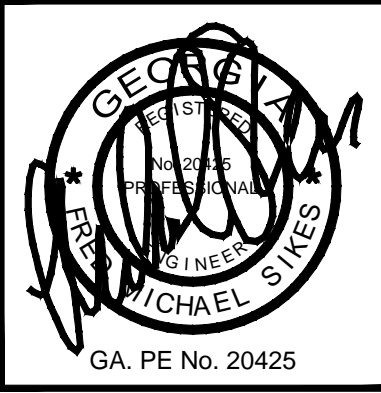
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1
S2.0
FIRST FLOOR FRAMING PLAN
SCALE: 3/16"=1'-0"

- FIRST FLOOR FRAMING PLAN NOTES:**
- TOP OF PRE-CAST HOLLOW CORE PLANKS 10'-8" AFF UNLESS NOTED OTHERWISE.
 - PROVIDE 2" MINIMUM CONCRETE TOPPING SLAB ($f_c = 4,000$ PSI AT 28 DAYS) REINFORCED WITH WWF 6x6xW2.1xW2.1 IN FLAT SHEETS. OVER ALL PRE-CAST HOLLOW CORE PLANKS.
 - PRECAST MEMBERS SHALL BE CAMBERED FOR THEIR DEAD LOAD PLUS 5 PSF.
 - VERIFY ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS.
 - SEE SHEET S0.4 FOR TYPICAL BEAM FRAMING CONNECTIONS.
 - PROVIDE THE FOLLOWING BEAM TO CMU CONNECTIONS:
P.T. 4x12 SIMPSON HUCQ412-SDS
P.T. 6x12 SIMPSON HUCQ612-SDS

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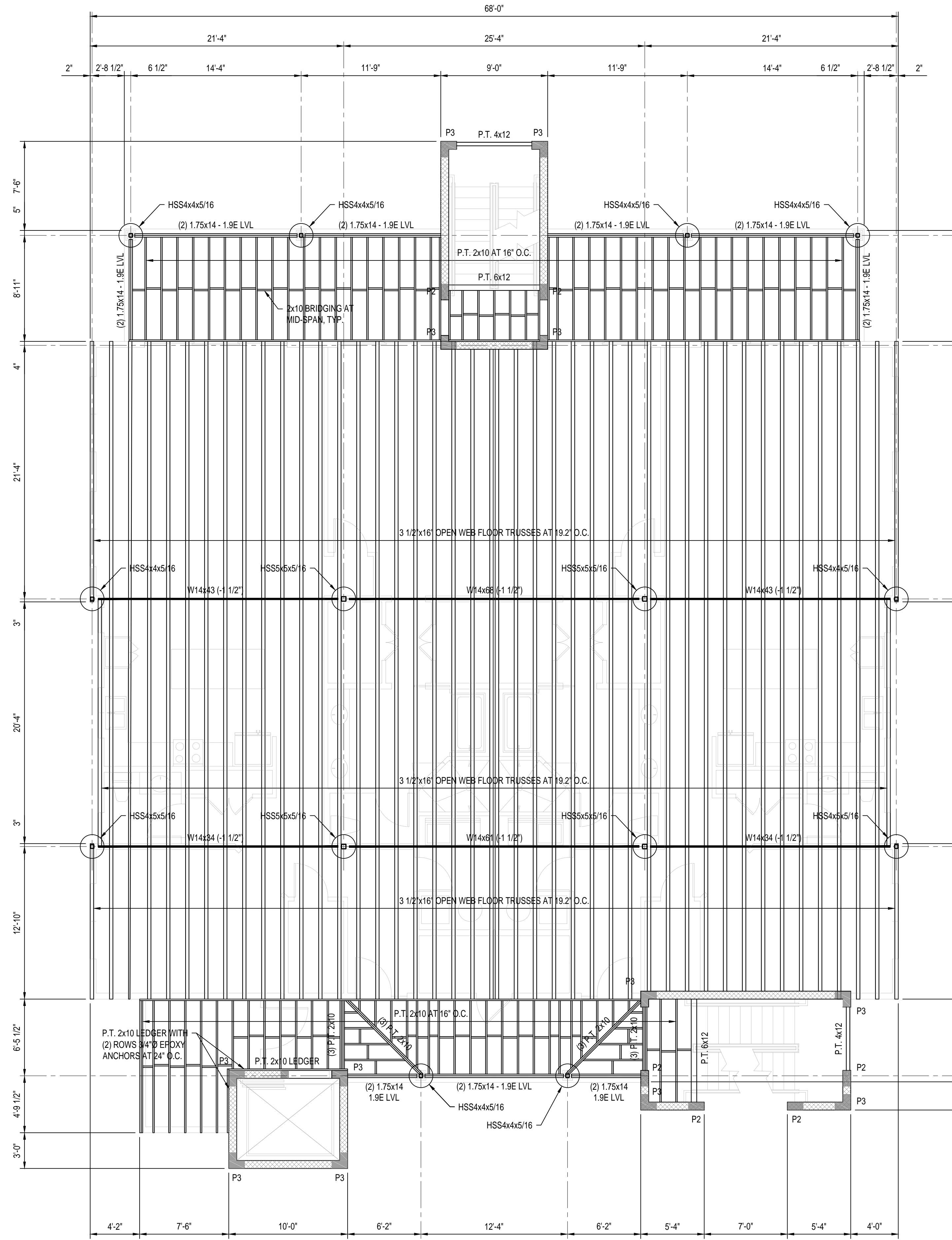
S2.0

DOOR AND WINDOW HEADER SCHEDULE

OPENING WIDTH	2x6 LOAD BEARING WALLS		2x6 NON-LOAD BEARING WALLS		2x6 NON-LOAD BEARING WALLS	
	HEADER	JACK STUDS EACH END	HEADER	JACK STUDS EACH END	HEADER	JACK STUDS EACH END
UP TO 6'-0"	(3) 2x10 SYP #2	1	(3) 2x8 SYP #2	1	(2) 2x8 SYP #2	1
OVER 6'-0" THRU 10'-0"	(3) 2x12 SYP #2	2	(3) 2x10 SYP #2	2	(2) 2x10 SYP #2	2
OVER 10'-0" THRU 12'-0"	(3) 1.75x14 - 1.9E LVL	3	(3) 2x12 SYP #2	2	(2) 2x12 SYP #2	2

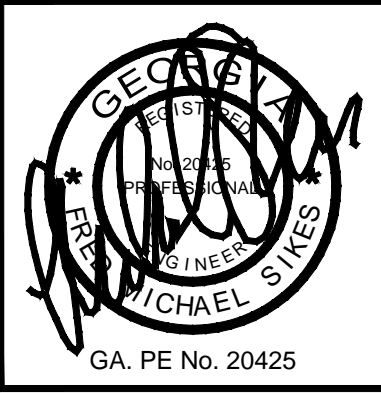
NOTES:

- PROVIDE 1/2" SOLID PLYWOOD FLITCH PLS. SAME DEPTH AS HEADER BETWEEN ALL 2x'S.
- WHERE SPANS EXCEED 8'-0" PROVIDE CONT. 1/2"x8" PLYWOOD FLITCH PL. CENTERED ON THE SPAN. FILL REMAINING SPACE WITH 1/2" PLYWOOD FLITCHES. NO SPLICES ALLOWED IN MIDDLE 3' OF SPAN.
- DOUBLE JACK STUDS SHALL BE ATTACHED WITH (2) ROWS 10d COMMONS AT 12" O.C. TRIPLE JACK STUDS SHALL BE ATTACHED WITH (2) ROWS 10d COMMONS AT 12" O.C. EACH FACE.



1
S2.1
SECOND FLOOR FRAMING PLAN
SCALE: 3/16"=1'-0"

- SECOND FLOOR FRAMING NOTES:**
- TOP OF FLOOR TRUSSES 22'-0" AFF UNLESS NOTED OTHERWISE.
 - VERIFY ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS AND DETAILS.
 - WHERE MULTIPLE LVL'S ARE INDICATED ON PLAN, CONTRACTOR MAY PROVIDE A SINGLE LVL OF EQUAL SIZE AND STRENGTH.
 - ALL EXTERIOR WALLS SHALL BE CONSIDERED SHEAR WALLS AND COVERED WITH 1/2" APA RATED EXTERIOR SHEATHING. ALL EDGES SHALL BE BLOCKED. SEE EXTERIOR WALL NAILING DIAGRAM FOR NAILING PATTERN.
 - ALL DOOR AND WINDOW HEADERS SHALL BE AS INDICATED IN THE SCHEDULE UNLESS SPECIFICALLY NOTED OTHERWISE ON FRAMING PLANS.
 - SEE SHEET S0.4 FOR TYPICAL BEAM FRAMING CONNECTIONS.
 - PROVIDE THE FOLLOWING BEAM TO CMU CONNECTIONS:
P.T. 4x12 SIMPSON HUCQ412-SDS
P.T. 6x12 SIMPSON HUCQ612-SDS



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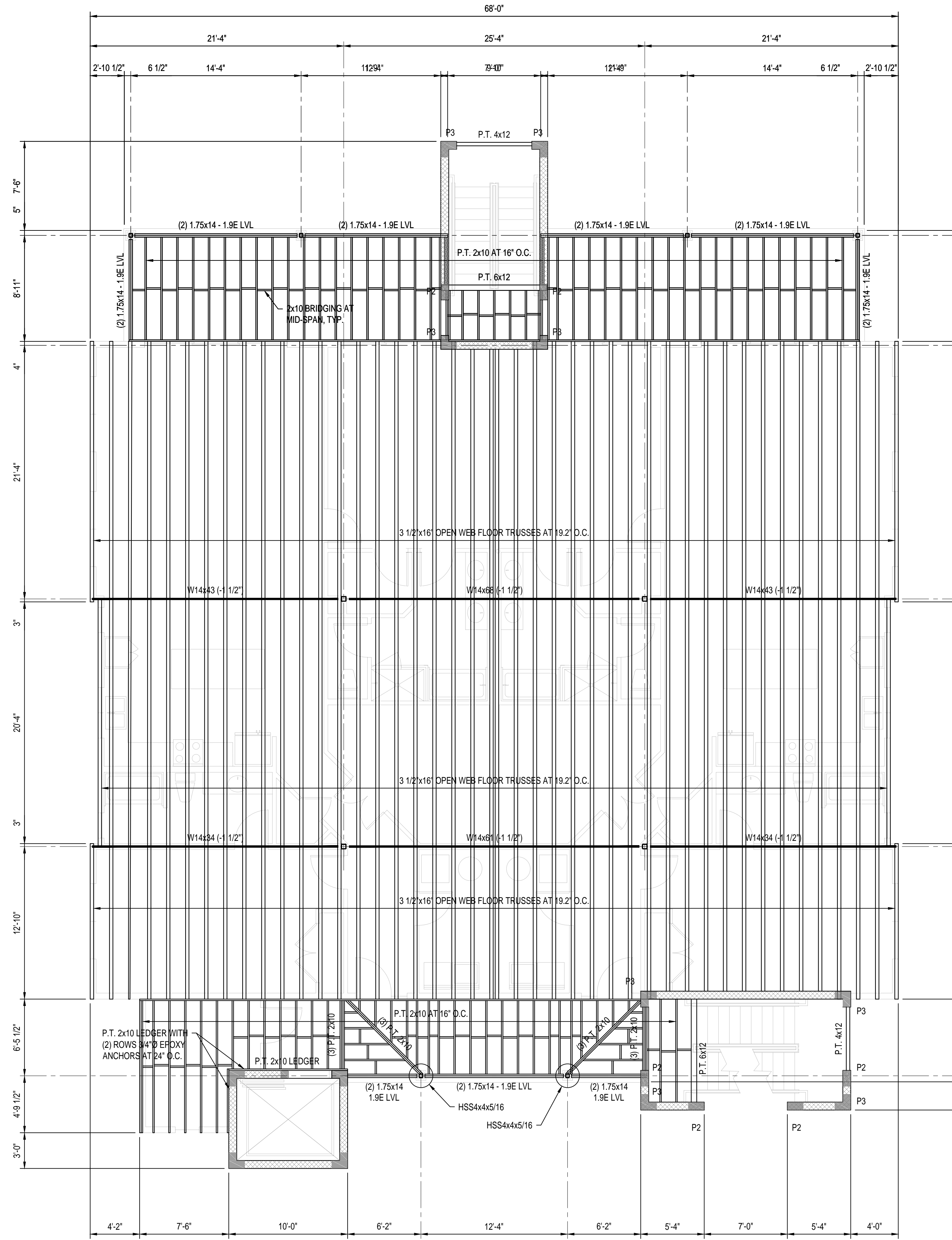
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DOOR AND WINDOW HEADER SCHEDULE

OPENING WIDTH	2x6 LOAD BEARING WALLS		2x6 NON-LOAD BEARING WALLS		2x6 NON-LOAD BEARING WALLS	
	HEADER	JACK STUDS EACH END	HEADER	JACK STUDS EACH END	HEADER	JACK STUDS EACH END
UP TO 6'-0"	(3) 2x10 SYP #2	1	(3) 2x8 SYP #2	1	(2) 2x8 SYP #2	1
OVER 6'-0" THRU 10'-0"	(3) 2x12 SYP #2	2	(3) 2x10 SYP #2	2	(2) 2x10 SYP #2	2
OVER 10'-0" THRU 12'-0"	(3) 1.75x16 - 1.9E LVL	3	(3) 2x12 SYP #2	2	(2) 2x12 SYP #2	2

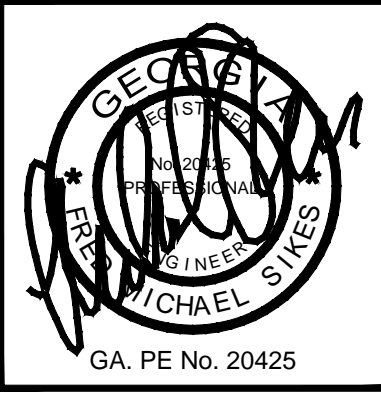
NOTES:

- PROVIDE 1/2" SOLID PLYWOOD FLITCH PLS. SAME DEPTH AS HEADER BETWEEN ALL 2xS.
- WHERE SPANS EXCEED 8'-0" PROVIDE CONT. 1/2x8" PLYWOOD FLITCH PL. CENTERED ON THE SPAN. FILL REMAINING SPACE WITH 1/2" PLYWOOD FLITCHES. NO SPLICES ALLOWED IN MIDDLE 8' OF SPAN.
- DOUBLE JACK STUDS SHALL BE ATTACHED WITH (2) ROWS 10d COMMONS AT 12" O.C. TRIPLE JACK STUDS SHALL BE ATTACHED WITH (2) ROWS 10d COMMONS AT 12" O.C. EACH FACE.



1
S2.2
THIRD FLOOR FRAMING PLAN
SCALE: 3/16"=1'-0"

- THIRD FLOOR FRAMING NOTES:**
- TOP OF FLOOR TRUSSES 33'-2 3/4" AFF UNLESS NOTED OTHERWISE.
 - VERIFY ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS AND DETAILS.
 - WHERE MULTIPLE LVL'S ARE INDICATED ON PLAN, CONTRACTOR MAY PROVIDE A SINGLE LVL OF EQUAL SIZE AND STRENGTH.
 - ALL EXTERIOR WALLS SHALL BE CONSIDERED SHEAR WALLS AND COVERED WITH 1/2" APA RATED EXTERIOR SHEATHING. ALL EDGES SHALL BE BLOCKED. SEE EXTERIOR WALL NAILING DIAGRAM FOR NAILING PATTERN.
 - ALL DOOR AND WINDOW HEADERS SHALL BE AS INDICATED IN THE SCHEDULE UNLESS SPECIFICALLY NOTED OTHERWISE ON FRAMING PLANS.
 - SEE SHEET S0.4 FOR TYPICAL BEAM FRAMING CONNECTIONS.
 - PROVIDE THE FOLLOWING BEAM TO CMU CONNECTIONS:
P.T. 4x12 SIMPSON HUCQ412-SDS
P.T. 6x12 SIMPSON HUCQ612-SDS



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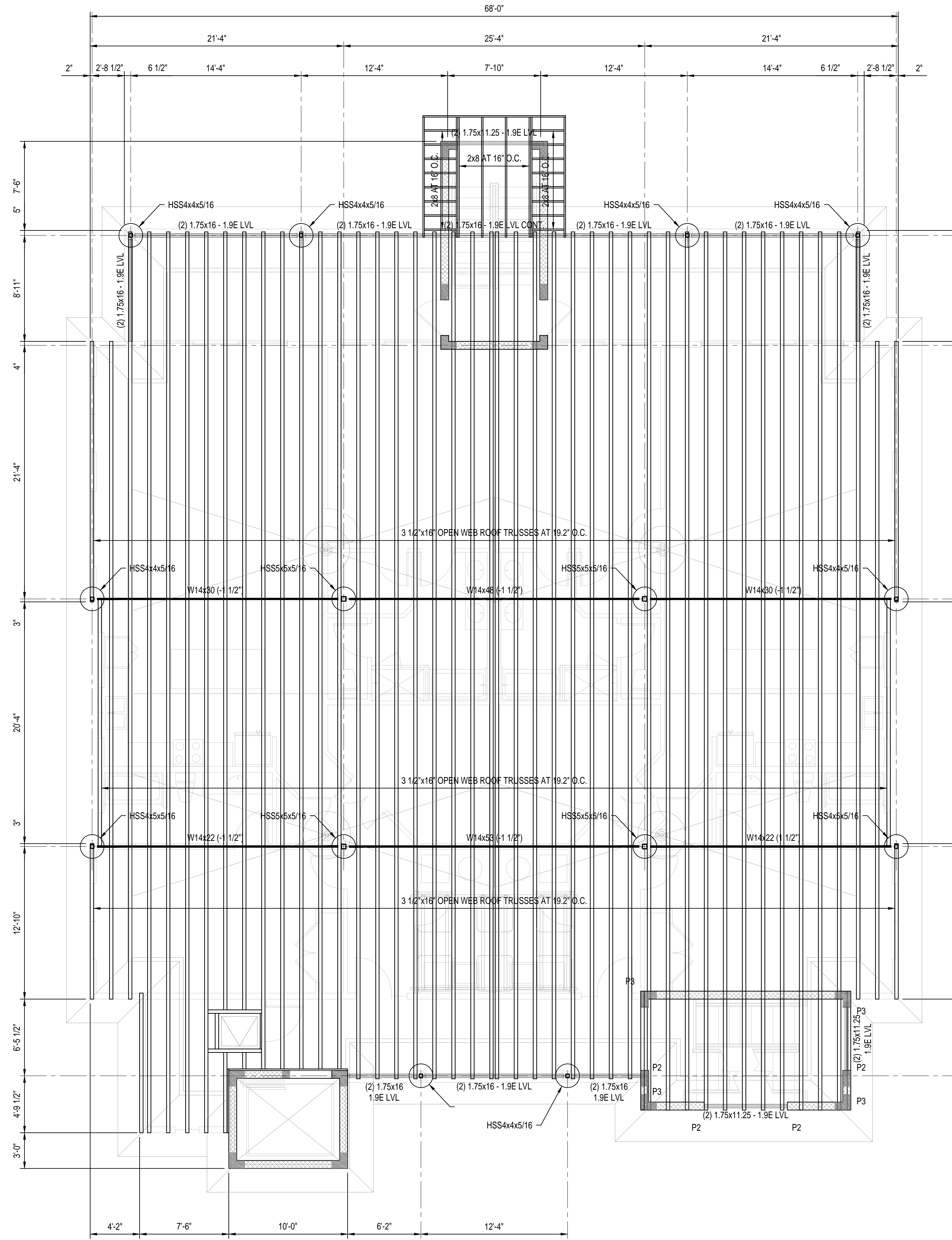
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DOOR AND WINDOW HEADER SCHEDULE

OPENING WIDTH	2x6 LOAD BEARING WALLS		2x6 NON-LOAD BEARING WALLS		2x6 NON-LOAD BEARING WALLS	
	HEADER	JACK STUDS EACH END	HEADER	JACK STUDS EACH END	HEADER	JACK STUDS EACH END
UP TO 6'-0"	(3) 2x10 SYP #2	1	(3) 2x8 SYP #2	1	(2) 2x8 SYP #2	1
OVER 6'-0" THRU 10'-0"	(3) 2x12 SYP #2	2	(3) 2x10 SYP #2	2	(2) 2x10 SYP #2	2
OVER 10'-0" THRU 12'-0"	(3) 1.75x16 - 1.9E LVL	3	(3) 2x12 SYP #2	2	(2) 2x12 SYP #2	2

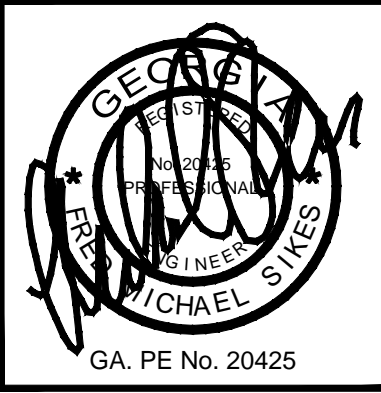
NOTES:

- PROVIDE 1/2" SOLID PLYWOOD FLITCH PLS. SAME DEPTH AS HEADER BETWEEN ALL 2xS.
- WHERE SPANS EXCEED 8'-0" PROVIDE CONT. 1/2x8" PLYWOOD FLITCH PL. CENTERED ON THE SPAN. FILL REMAINING SPACE WITH 1/2" PLYWOOD FLITCHES. NO SPLICES ALLOWED IN MIDDLE 3' OF SPAN.
- DOUBLE JACK STUDS SHALL BE ATTACHED WITH (2) ROWS 10# COMMONS AT 12" O.C. TRIPLE JACK STUDS SHALL BE ATTACHED WITH (2) ROWS 10# COMMONS AT 12" O.C. EACH FACE.



1
S2.3
ROOF FRAMING PLAN
SCALE: 3/16"=1'-0"

- ROOF FRAMING NOTES:**
- TOP OF ROOF TRUSSES 44'-5 1/2" AFF UNLESS NOTED OTHERWISE.
 - VERIFY ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS AND DETAILS.
 - WHERE MULTIPLE LVL'S ARE INDICATED ON PLAN, CONTRACTOR MAY PROVIDE A SINGLE LVL OF EQUAL SIZE AND STRENGTH.
 - ALL EXTERIOR WALLS SHALL BE CONSIDERED SHEAR WALLS AND COVERED WITH 1/2" APA RATED EXTERIOR SHEATHING. ALL EDGES SHALL BE BLOCKED. SEE EXTERIOR WALL NAILING DIAGRAM FOR NAILING PATTERN.
 - ALL DOOR AND WINDOW HEADERS SHALL BE AS INDICATED IN THE SCHEDULE UNLESS SPECIFICALLY NOTED OTHERWISE ON FRAMING PLANS.
 - SEE SHEET S0.4 FOR TYPICAL BEAM FRAMING CONNECTIONS.
 - PROVIDE THE FOLLOWING BEAM TO CMU CONNECTIONS:
(2) 1.75x11.25 LVL SIMPSON HUCQ412-SDS



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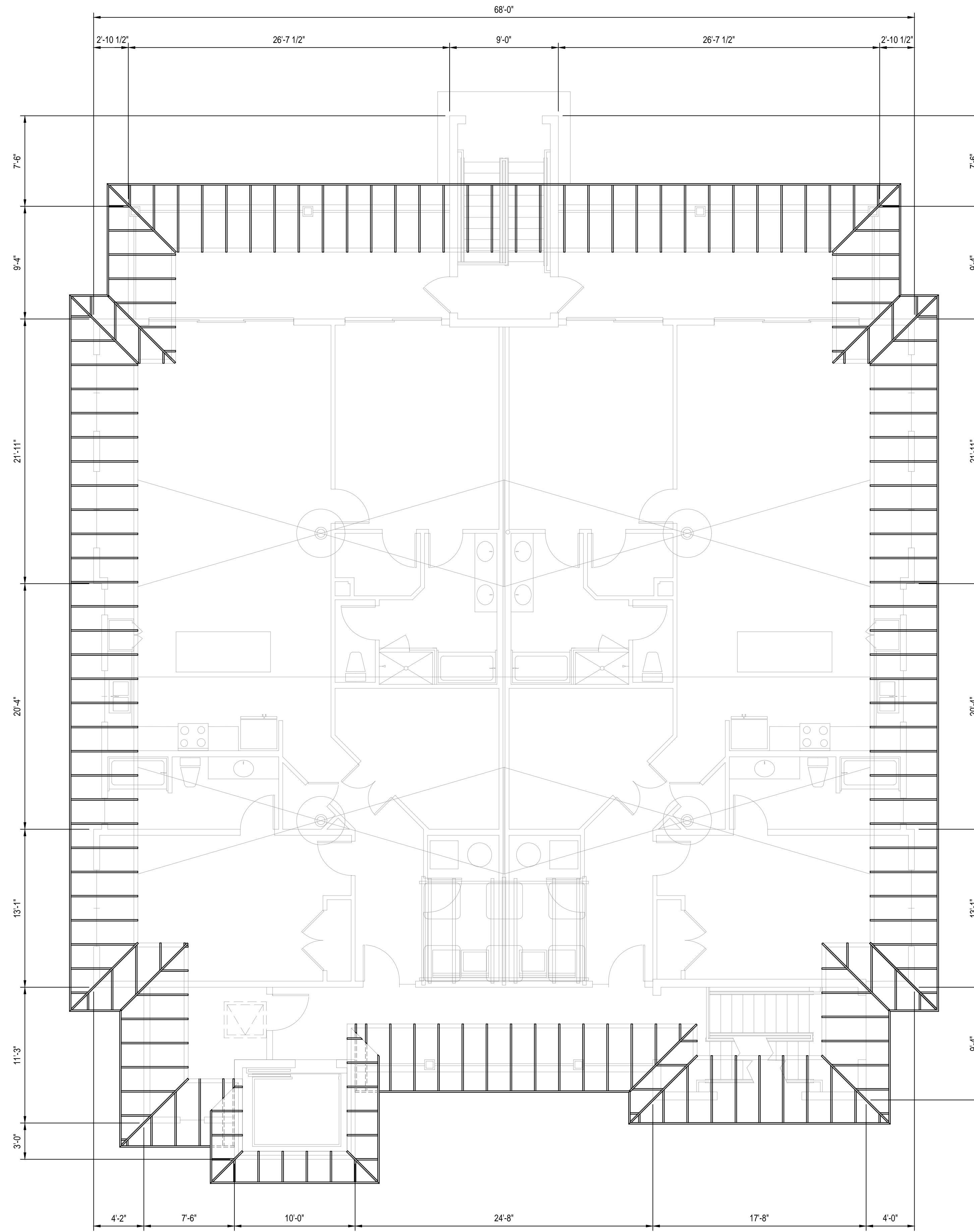
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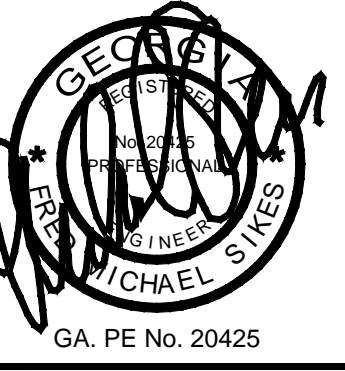
F. MICHAEL SIKES, P.E., LLC
Structural Engineer
54 Rangier Road Savannah, Georgia 31419
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1
S2.4 ROOF EYE-BROW FRAMING PLAN
SCALE: 3/16"=1'-0"

- ROOF EYE-BROW FRAMING NOTES:**
1. VERIFY ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS AND DETAILS.
 2. EYE-BROW FRAMING SHALL BE PRE-ENGINEERED WOOD TRUSSES AT 24" O.C. UNLESS NOTED OTHERWISE.

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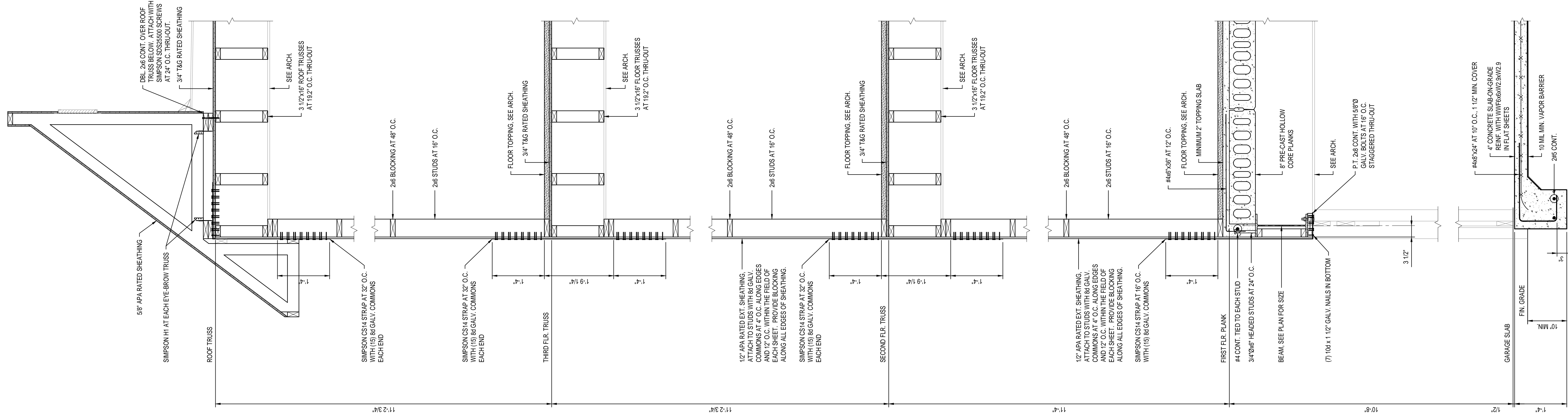
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A
TYPICAL WALL SECTION
SCALE: 3/4"=1'-0"

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